# **Appendix A** — Arkadelphia Bypass Traffic Study

# Arkadelphia Bypass Traffic Study

HWY. 67 TO HWY. 51 (ARKADELPHIA BYPASS) P.E.

ARDOT Job 070442; F.A.P. Number STPC-9013(10)

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#### **List of Appendices**

Appendix A - Volume Data

**Appendix B – Intersection Operational Analysis Results** 

**Appendix C – Roadway Segment Operational Analysis Results** 

Due to the size of these appendices (1,145 pages in total), they have been removed from this report but are available upon request.

## INTRODUCTION

Arkadelphia is adjacent to the only Ouachita River crossing in the region, and this route (Highway 51) continues through the heart of Arkadelphia and provides critical access to Highway 67 and Interstate 30 (I-30). A number of sawmills operate within 40 miles of Arkadelphia, and trucks going to and from the sawmills utilize this river crossing. These trucks must negotiate several difficult turns and pass through the Arkadelphia Central Business District (CBD), which imposes delays on other road users within the city.

In 2013, the Arkansas Department of Transportation (ARDOT) conducted a study to investigate alternate bridge locations and bypass options to better serve the heavy truck traffic and long distance through traffic. Since then, the existing bridge has been replaced at the same location, and no bypass has been constructed.

As part of the NEPA process, Garver reviewed the previous study and is providing an updated traffic and safety study which includes an analysis of the existing and future No Action conditions, safety analysis, and analysis of the new proposed bypass alternatives.

Figure 1 on the following page shows the study area. The following sections of this report document the findings of the traffic and safety analyses.

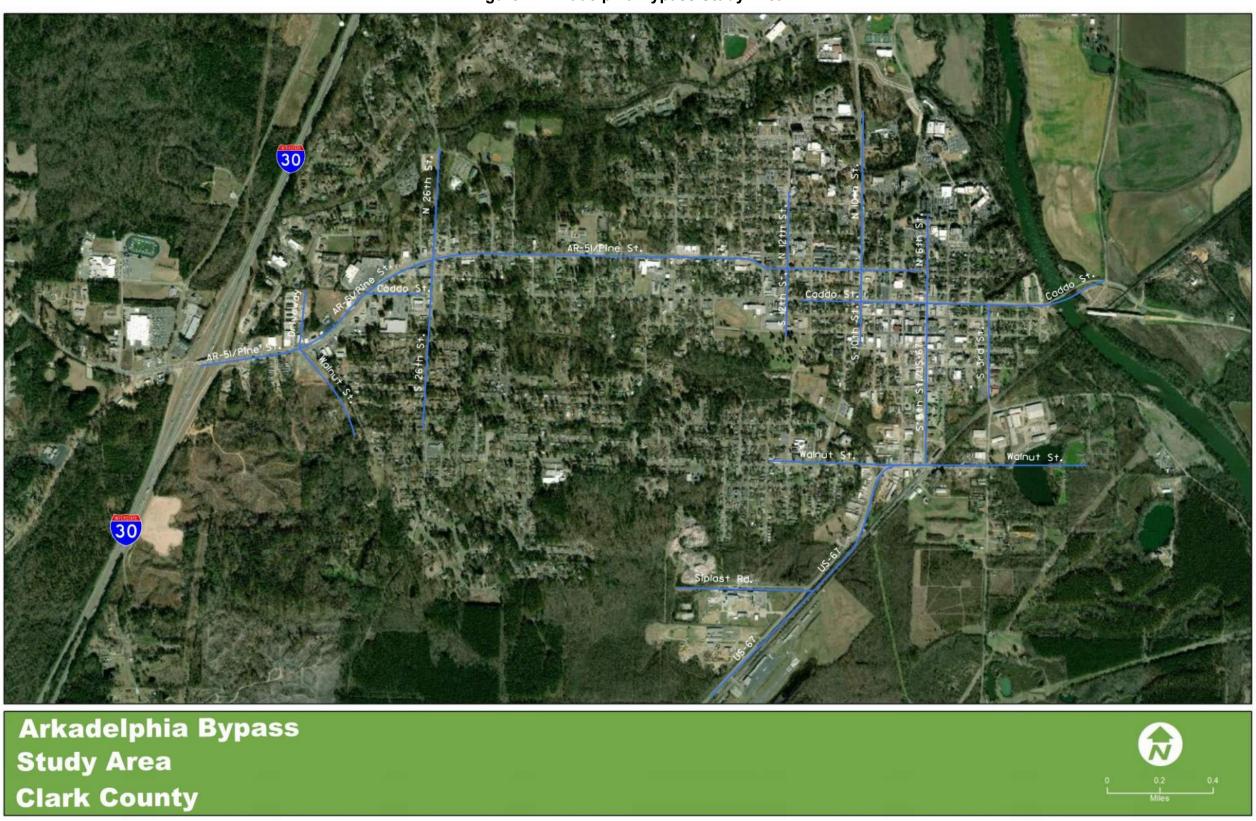


Figure 1: Arkadelphia Bypass Study Area

Arkadelphia Bypass Traffic Study

## **EXISTING / NO ACTION ANALYSIS**

An updated analyses of existing and future No Action conditions were conducted to identify any safety or operational issues and to provide a basis of comparison for the proposed Action alternatives. The results of the existing and future No Action analyses are described in the following subsections.

#### **2018 EXISTING VOLUMES**

In April of 2018, turning movement counts were conducted at key intersections within the study area and included 24-hours of data with percentages of buses and trucks. This data was processed to identify the peak hours, peak hour factors, and percentages of heavy vehicles during the peak hours. For this study area, a single seasonal adjustment factor of 0.98 was applied according to the methodologies outlined in the ARDOT *Traffic Monitoring System Handbook* for an urban principal arterial with data collected in April. No balancing was required between intersections due to the presence of cross streets and driveways between the study intersections.

After the initial volumes were developed, the study area was expanded further west along Pine Street. Additional turning movement counts were collected at key intersections within the added area in September and October of 2018. Seasonal adjustment factors of 0.95 and 0.99 were applied to the October and September counts, respectively, in accordance with the ARDOT *Traffic Monitoring System Handbook* for an urban principal arterial with data collected in these months. As with the original study area, no balancing was required between intersections for the expanded study area due to the presence of cross streets and driveways between the study intersections. The resulting 2018 Existing design volumes are shown in **Figure 2** and in **Figure 3** on the following pages. The raw turning movement data is provided in **Appendix A - Volume Data**.

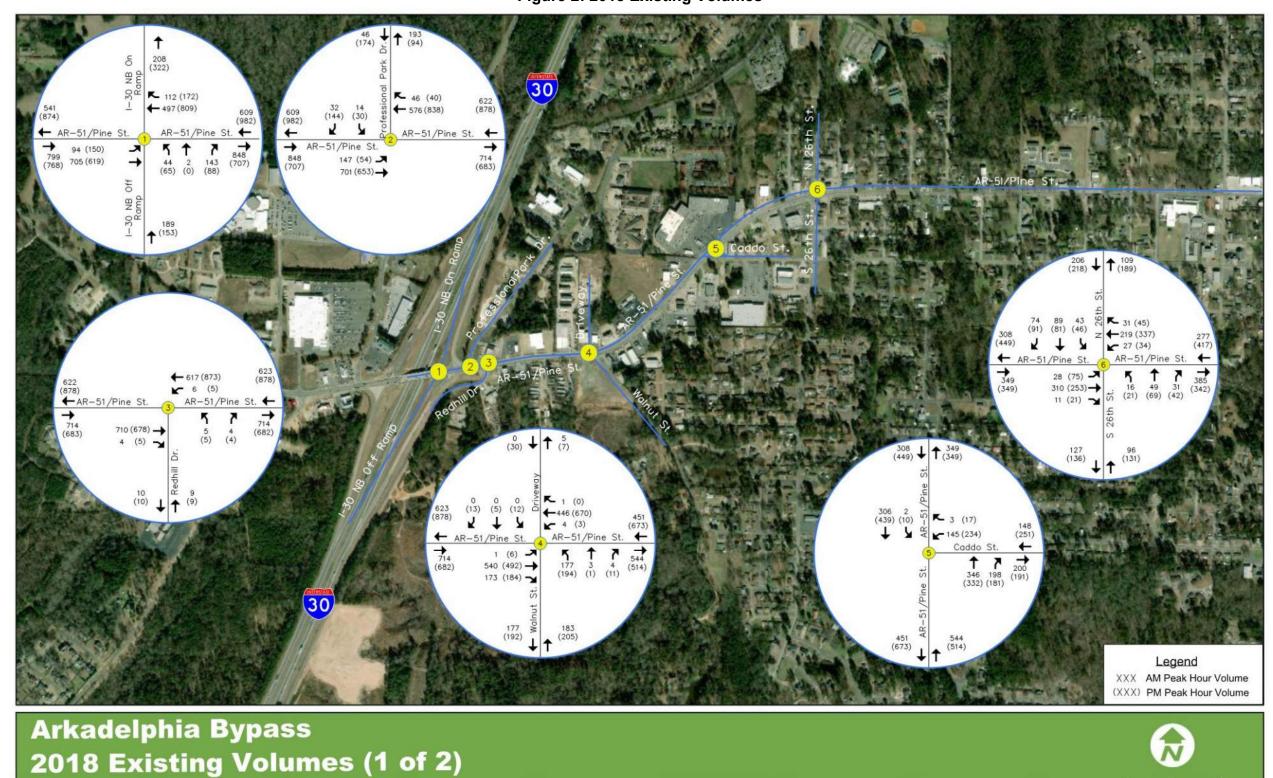


Figure 2: 2018 Existing Volumes

Arkadelphia Bypass Traffic Study

**Clark County** 

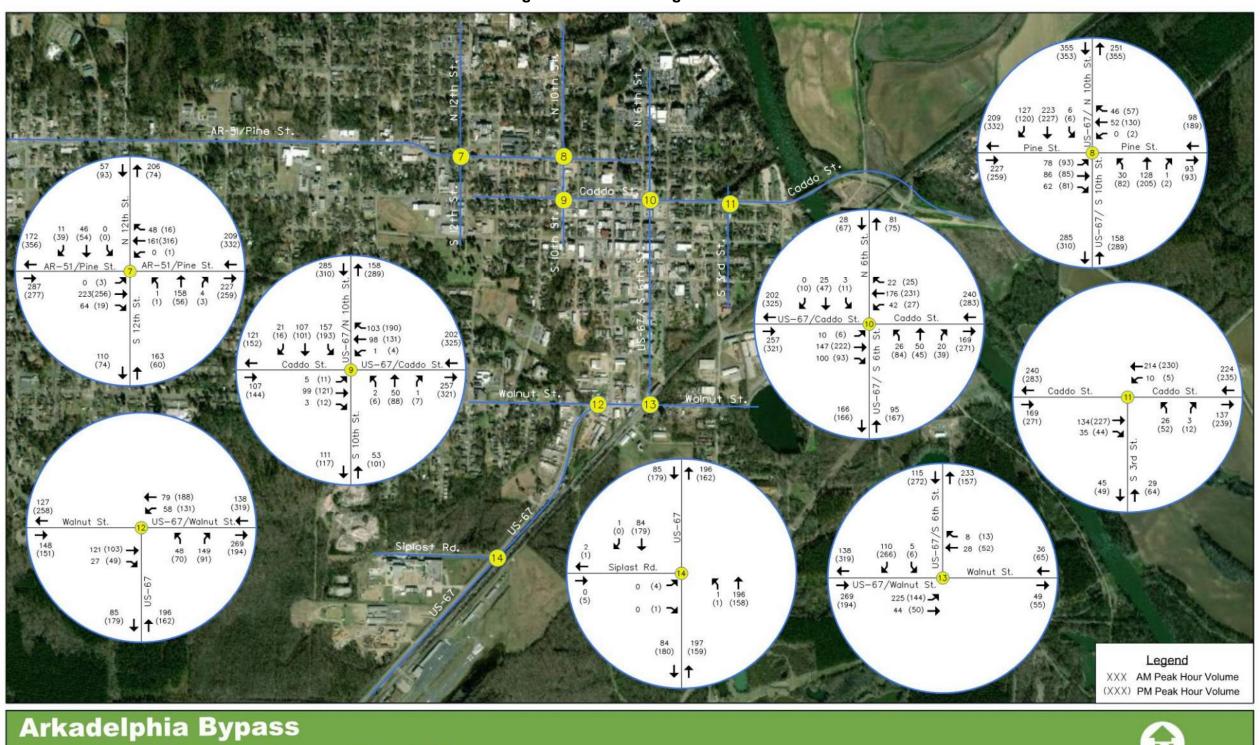


Figure 3: 2018 Existing Volumes

**Arkadelphia Bypass Traffic Study** 

**Clark County** 

2018 Existing Volumes (2 of 2)

#### TRAFFIC FORECAST

From the ARDOT database, fourteen ADT counting stations were identified near or within the study area. All of these stations had over twenty years of data. The historic data was compiled into an Excel sheet and is also shown in **Table 1** on the following page.

Several methods were investigated to determine the growth rate and project future volumes. First, the trend function was used in Excel to project 2038 traffic volumes based on the historic volumes. This function is based on the equation y=mx+b, where y represents the traffic volume and x represents the year. For these calculations, the true "b" value was selected. The projected volumes and resulting growth rates based on the trend function are shown in **Table 2**. Negative growth rates were not considered in determining the appropriate growth rate to use for this traffic study, and all growth rates not utilized are shaded a darker gray.

**Table 1: Historic ADT Data** 

Location	Hwy. 51 East of Ouachita River	Caddo St. (Hwy. 51) - b/t 3 <sup>rd</sup> St. and 4 <sup>th</sup> St.	Caddo St. (Hwy. 67) - b/t 8 <sup>th</sup> St. and 9 <sup>th</sup> St.	10th St. (Hwy. 67) - b/t Pine St. & Caddo St.	Pine St. (Hwy. 51) - b/t Robey St. & 11 <sup>th</sup> St.	Pine St. (Hwy. 51) - b/t Hunter St. & 13th St.	Pine St. (Hwy. 51) - b/t 20th St. & 19th St.	Pine St. (Hw y. 51) - b/t 27th St. & 26th St.	Pine St. (Hwy. 51) - b/t Walnut St. & 29th St.	Pine St. (Hwy. 51) - b/t Red Hill Rd. & Walnut St.	6th St. (Hwy. 67) - b/t Crittende n St. & Carpenter St.	Walnut St. - b/t 11 <sup>th</sup> St. & Hwy. 67	Walnut St. b/t 13 <sup>th</sup> St. & 12 <sup>th</sup> St.	Hwy. 67 - South of Walnut St.
Station	100028	100208	100211	100212	100207	100206	100205	100204	100203	100202	100210	10S096	10S095	100209
1998	3,200	3,000	6,900	8,100	8,300	11,000	11,000	11,000	12,000	14,000	2,500	2,800	3,900	4,100
1999	2,800	4,200	6,300	7,000	6,300	8,400	8,200	8,500	10,000	13,500	2,500	3,050	3,300	4,200
2000	2,800	3,600	6,100	6,900	6,000	8,000	7,300	8,700	11,000	13,000	2,600	3,300	4,500	3,500
2001	3,200	4,100	6,900	7,900	7,300	9,100	9,800	10,000	13,000	15,000	2,800	3,700	4,700	4,200
2002	2,900	4,200	6,200	5,600	6,200	7,400	8,000	8,600	11,000	13,000	2,800	3,400	4,800	3,700
2003	3,000	4,600	6,600	7,400	6,600	8,100	9,000	9,900	13,000	15,000	2,800	4,000	4,800	3,900
2004	3,500	4,800	6,700	7,300	6,200	7,900	8,300	9,600	12,300	14,400	2,600	3,600	5,400	4,200
2005	3,000	4,550	6,400	7,400	7,900	9,200	10,300	10,600	13,300	16,000	2,950	3,800	5,150	3,900
2006	3,300	4,300	7,300	7,600	7,500	9,500	11,100	12,000	16,100	18,800	3,300	4,100	4,900	4,200
2007	3,100	4,000	7,300	7,900	8,100	10,400	10,800	11,400	14,100	17,200	2,900	4,000	5,000	4,200
2008	2,900	4,400	7,000	7,400	7,500	8,000	9,800	10,000	13,000	16,000	2,900	3,700	5,000	3,500
2009	2,900	4,000	7,000	7,500	7,300	8,700	10,000	13,000	14,000	18,000	2,800	3,700	5,000	3,600
2010	3,000	4,500	7,200	7,700	7,600	9,000	11,000	13,000	15,000	18,000	3,000	3,700	5,200	3,600
2011	3,000	4,700	7,700	8,300	7,700	10,000	9,000	9,800	15,000	16,000	3,200	3,600	5,200	4,100
2012	3,000	4,700	7,400	7,800	7,400	7,600	9,700	9,300	12,000	17,000	2,900	3,600	5,300	3,600
2013	2,800	5,300	10,000	11,000	7,800	11,000	9,800	12,000	16,000	17,000	2,800	3,700	5,000	3,300
2014	3,100	5,600	8,800	8,400	7,600	8,700	9,700	11,000	13,000	17,000	3,200	3,700	4,700	3,600
2015	2,300	5,200	7,700	7,800	7,300	8,500	9,400	10,000	13,000	17,000	2,800	3,300	4,800	3,600
2016	3,100	4,800	7,500	7,500	7,100	8,700	9,000	9,900	13,000	17,000	2,800	3,300	4,800	3,500
2017	2,700	4,800	7,500	9,700	6,700	8,100	8,700	10,000	13,000	18,000	2,900	3,500	4,800	3,800
2018	3,000	4,800	7,800	7,700	7,400	8,300	9,400	11,000	14,000	17,000	2,800	3,200	4,700	3,400

Table 2: AGR based on the Trend Function

Location	East of Ouachita	Caddo St. (Hw y. 51) - b/t 3 <sup>rd</sup> St. and 4 <sup>th</sup> St.	(Hw y. 67) - b/t 8 <sup>th</sup> St.	b/t Pine	_	Pine St. (Hwy. 51) - b/t Hunter St. & 13th St.	b/t 20th	b/t 27th	b/t Walnut	(Hwy. 51) - b/t Red Hill Rd. &	b/t Crittende	- b/t 11 <sup>th</sup> St. & Hwy.	Walnut St. b/t 13 <sup>th</sup> St.	Hwy. 67 - South of Walnut St.	Average AGR (%)
AGR(%)	-0.68	1.57	1.35	1.46	0.43	0.15	0.33	0.57	0.90	1.37	0.84	0.74	1.05	-0.73	
2018	3,000	4,800	7,800	7,700	7,400	8,300	9,400	11,000	14,000	17,000	2,800	3,200	4,700	3,400	0.90
2038	2,615	6,550	10,190	10,287	8,062	8,546	10,044	12,313	16,750	22,328	3,312	3,710	5,795	2,938	

Next, future volumes were projected using the growth rate calculated based on **Equation 1**.

**Equation 1:**  $V_F = V_P^* GF^n$ 

GF = (1 + AGR/100)

Where:  $V_F = \text{future volume}$ 

 $V_P$  = present volume

GF = growth factor

AGR = annual growth rate (%)

n = number of years

The annual growth rate was calculated based on the 1998 and 2018 ADT for each station as shown in **Table 3** on the following page. Negative growth rates were not considered in determining the appropriate growth rate to use for this traffic study, and all growth rates not utilized are shaded a dark gray.

**Table 3: Projected ADT Based on the Calculated Growth Rate** 

Location	East of Ouachita	Caddo St. (Hwy. 51) - b/t 3 <sup>rd</sup> St. and 4 <sup>th</sup> St.	(Hwy. 67) - b/t 8 <sup>th</sup> St.	10th St. (Hwy. 67) - b/t Pine St. & Caddo St.	St. & 11 <sup>th</sup>	b/t Hunter		b/t 27th	b/t Walnut	b/t Red Hill Rd. &	Crittende	- b/t 11 <sup>th</sup> St. & Hwy. 67	b/t 13 <sup>th</sup> St.	Hwy. 67 - South of Walnut St.	Average AGR (%)
AGR(%)	-0.32	2.38	0.61	-0.25	-0.57	-1.40	-0.78	0.00	0.77	0.98	0.57	0.67	0.94	-0.93	
2018	3,000	4,800	7,800	7,700	7,400	8,300	9,400	11,000	14,000	17,000	2,800	3,200	4,700	3,400	0.66
2038	2,813	7,680	8,817	7,320	6,598	6,263	8,033	11,000	16,333	20,643	3,136	3,657	5,664	2,820	

Next, ARDOT calculates county growth rates across the state. Based on this information provided by ARDOT, a growth factor of 1.198 (AGR = 0.90%) for Clark County was applied to all of the stations within the study area as shown in **Table 4**.

**Table 4: Projected ADT Based on County Growth Rate** 

Location	Hwy. 51 East of Ouachita River	(Hw y. 51) - b/t 3 <sup>rd</sup> St.	Caddo St. (Hwy. 67) - b/t 8 <sup>th</sup> St. and 9 <sup>th</sup> St.	C+ 2.	b/t Robey	Pine St. (Hwy. 51) - b/t Hunter St. & 13th St.		b/t 27th	b/t Walnut St. & 29th	(Hwy. 51) - b/t Red	b/t Crittende n St. &	- b/t 11 <sup>th</sup> St. & Hwy.	Walnut St. b/t 13 <sup>th</sup> St. & 12 <sup>th</sup> St.	Hwy. 67 - South of Walnut St.	Average AGR (%)
AGR(%)							0.	90							0.90
2018	3,000	4,800	7,800	7,700	7,400	8,300	9,400	11,000	14,000	17,000	2,800	3,200	4,700	3,400	
2038	3,589	5,742	9,331	9,211	8,852	9,929	11,245	13,159	16,748	20,336	3,350	3,828	5,622	4,067	

Volumes and growth rates developed in the previous study used similar methodology and are shown in **Table 5**. They were reviewed for comparison but were not included in the tabulation for this study. Compared to the current study, the previous study utilized a much smaller growth rate.

**Table 5: Projected ADT Based on Previous Study** 

Location	East of Ouachita	Caddo St. (Hw y. 51) - b/t 3 <sup>rd</sup> St. and 4 <sup>th</sup> St.	(Hw y. 67) - b/t 8 <sup>th</sup> St.	b/t Pine	St. & 11 <sup>th</sup>	b/t Hunter	b/t 20th	b/t 27th	b/t Walnut St. & 29th	b/t Red Hill Rd. &	6 <sup>th</sup> St. (Hwy. 67) - b/t Crittende n St. & Carpenter St.	- b/t 11 <sup>th</sup>	b/t 13 <sup>th</sup> St.	Hwy. 67 - South of Walnut St.	Average AGR (%)
	Previous Studies Methodology														
2013	3,000		7,800	9,500	7,800						3,200	-	-	4,100	
2033	3,300		8,600	10,500	8,600						3,600	-	-	4,600	
AGR(%)	0.48	-	0.33	0.33	0.33						0.39	-	-	0.38	0.37

Note: Data from Previous Study was used for comparison, not as a data point in determining the AGR for this study.

**Table 6** shows a summary of the various calculated growth rates and the growth rate selected for this study. The negative growths shaded dark gray in the previous tables were not included when calculating the average growth rates for each of the methodologies. Based on these average growth rates for each methodology, a growth rate of 0.80% was selected to project future volumes throughout the study area for this traffic study.

**Table 6: Summary of Growth Rates** 

	Trend Function y-mx+b	Calculated V <sub>F</sub> =V <sub>P</sub> *GF	County	Average AGR (%)	Selected AGR (%)
Avg AGR (%)	0.90	0.66	0.90	0.82	0.80

#### **2040 NO ACTION VOLUMES**

The 2040 No Action Design volumes were projected from the 2018 Existing Design volumes using the exponential growth rate of 0.80% which was determined based on the traffic forecast described in the previous subsection. No balancing was required due to the presence of cross streets and driveways between each of the study intersections. The 2040 No Action Design volumes are shown in **Figure 4** and **in Figure 5** on the following pages.

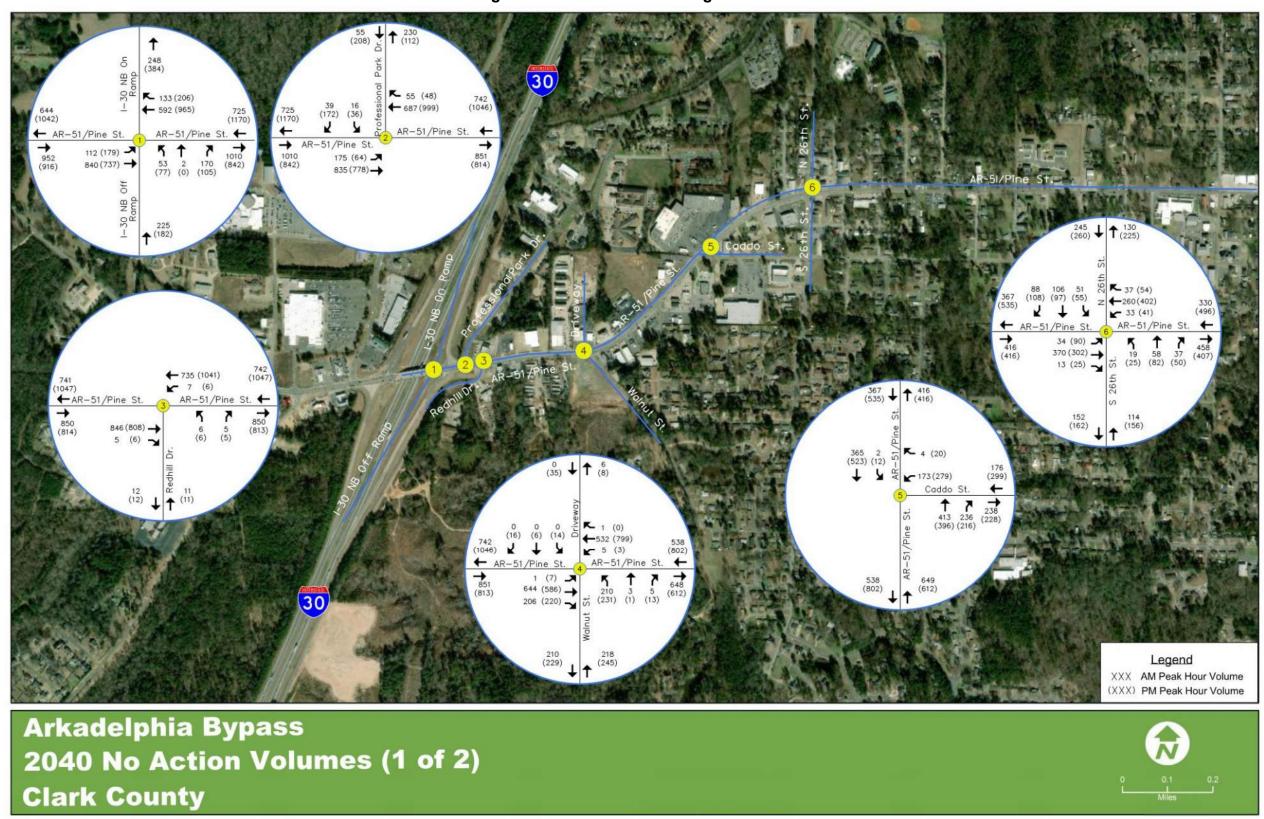


Figure 4: 2040 No Action Design Volumes

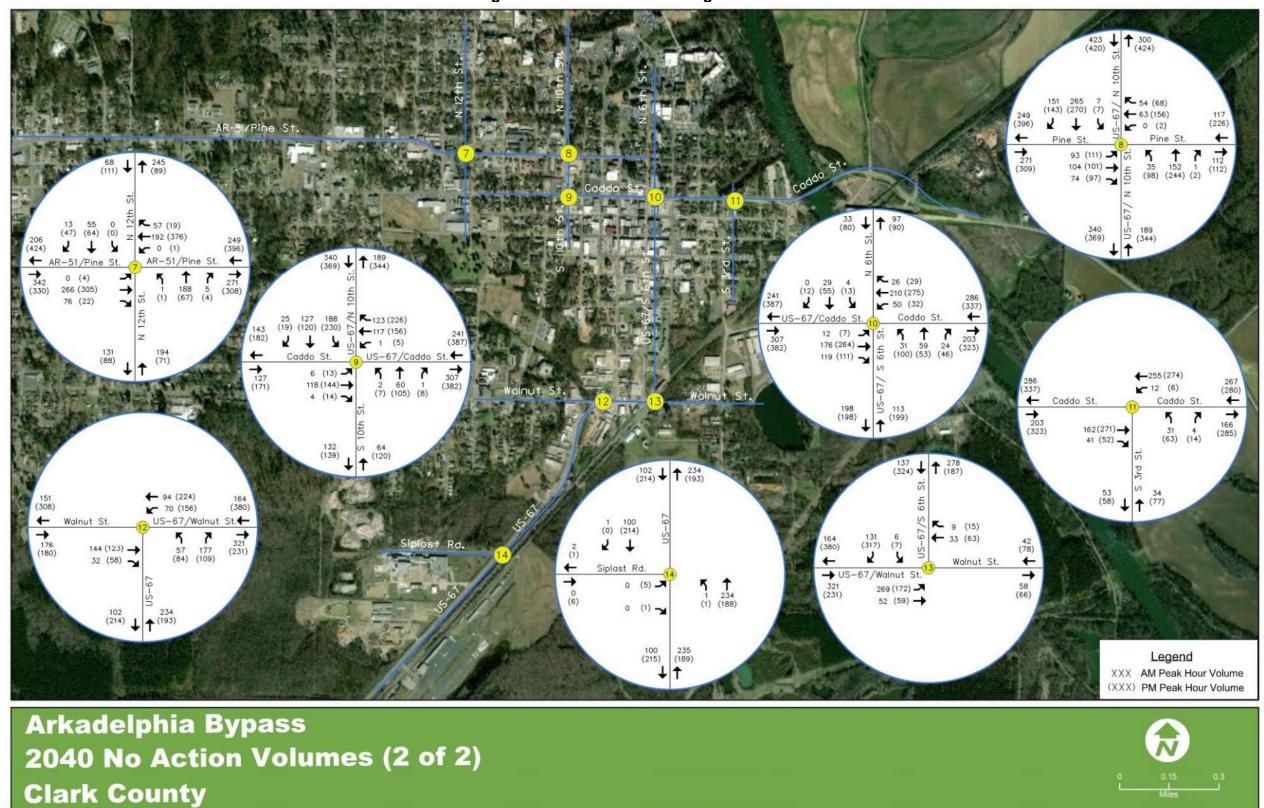
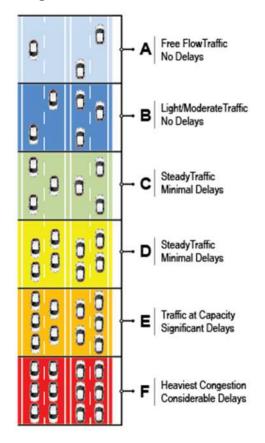


Figure 5: 2040 No Action Design Volumes

#### OPERATIONAL ANALYSIS METHODOLOGY

The 2018 Existing and 2040 No Action conditions were used to establish a baseline for comparison of the Action Alternatives with regards to congestion. Congestion along the project corridor was evaluated using Level of Service (LOS) as a performance measure. LOS is a qualitative measure used to depict operational conditions within a traffic stream or at an intersection. LOS is typically designated into six categories. These range from LOS A indicating free-flow, low density, or nearly negligible delay conditions to LOS F where demand exceeds capacity and large queues are experienced. A graphical representation of LOS is presented in **Figure 6**.

Figure 6: Level of Service (LOS) Categories



#### **Intersection Analysis Methodology**

Operational analyses of the intersections were conducted using *Synchro 10* and *SimTraffic* software according to the *Highway Capacity Manual 6<sup>th</sup> Edition (HCM)* methodology and *SimTraffic* microsimulation methodology. Microsimulation allows the user to analyze intersection operations both individually and in context of the entire study network. Additionally, microsimulation gives the user a powerful visualization tool to trace any sources of vehicle delay and queuing as well as the opportunity to perform multiple simulation runs with varying traffic loading within the peak hour to account for the expected variability within a system. This variation also accounts for the various types of drivers (aggressiveness, gap acceptance tolerance) and vehicles (performance on grades,

general acceleration /deceleration). Finally, micro-simulation provides the best means to demonstrate the impacts of queues on nearby intersections.

For intersections, the *HCM* uses control delay, measured in average seconds of delay per vehicle, as the basis for determining LOS. Control delay at an intersection is the average stopped time per vehicle traveling through the intersection plus the movements at slower speeds due to the vehicles moving up in the queue or slowing upstream of the approach. **Table 7** shows the LOS delay thresholds as stated in *HCM*, pages 19-16, 20-6 and 21-9.

Table 7: LOS Thresholds for Intersections (Control Delay)

Level of Service	Signal Control Control Delay Range (sec/veh)	Stop Control Control Delay Range (sec/veh)
Α	0 to 10	0 to 10
В	>10 to 20	>10 to 15
С	>20 to 35	>15 to 25
D	>35 to 55	>25 to 35
E	>55 to 80	>35 to 50
F	>80	>50

#### **Roadway Segment Analysis Methodology**

Operational analysis along the corridors was conducted using the two-lane highway module of the *Highway Capacity Software* (*HCS*), according to *HCM* methodology. The two-lane highway module within *HCS* was utilized to analyze the operating conditions of the roadway segments within the study area. For two-lane highways, the LOS is defined based on different criteria depending on the classification of the highway. Below is a brief description of each classification type:

#### Class I

- o Motorists expect relatively high speeds.
- Major intercity route, primary connector, commuter routes, etc.
- Serves mostly long-distance trips.

#### ➤ Class II

- Motorists do not expect to sustain high speeds.
- Access route to Class I facility, scenic/recreational route, or rugged terrain.
- Serves shorter trips, beginning/ending portions of longer trips, or sightseeing trips.

#### Class III

- Reduced speed limits.
- Higher number of unsignalized driveways and cross streets.
- Serves moderately developed areas; local traffic mixes with through traffic.

For Class I highways, the LOS is defined based on average travel speed (ATS) and percent time spent following (PTSF). For Class II highways, the LOS is defined based on PTSF. For Class III highways, the LOS is defined based on percent of free flow speed (PFFS). **Table 8** shows the LOS thresholds for each classification of two-lane highway as stated in the *HCM 6<sup>th</sup> Edition*, pg 15-8. Note that for this study, only Class I and Class III classifications were used.

Table 8: LOS Thresholds for Two-lane Highways Intersections

Level of	Cla	ss I	Class II	Class III							
Service	ATS (mi/h)	PTSF (%)	PTSF (%)	PFFS (%)							
Α	> 55	<u>≤</u> 35	<u>≤</u> 40	> 91.7							
В	> 50 - 55	> 35 - 50	> 40 - 55	> 83.3 – 91.7							
С	> 45 - 50	> 50 - 65	> 55 - 70	> 75.0 – 83.3							
D	> 40 - 45	> 65 - 80	> 70 - 85	> 66.7 – 75.0							
E	<u>≤</u> 40 > 80		> 85	<u>&lt;</u> 66.7							
F	Demand exceeds capacity										

#### **2018 EXISTING CONDITIONS**

Using the 2018 Existing volumes, the existing signalized intersections were modeled, and the existing signal timings were optimized using actuated control settings. An AM Peak Hour Factor (PHF) of 0.80 and a PM PHF of 0.86 were utilized. The percentage of heavy vehicles was modeled as 3% for both peak periods. The results of the operational analysis of existing conditions are reported in the following subsections.

#### **2018 Intersection Analysis, Existing Conditions**

The results based on *HCM* methodology and *SimTraffic* methodology are summarized in **Tables 9 and 10**. The complete results are provided in **Appendix B – Intersection Operational Analysis Results.** These results demonstrate that all movements at the study intersections operate acceptably under 2018 Existing conditions.

Table 9: Intersection Analysis – 2018 Existing Conditions – HCM

lute ve e eti e ve	Time	Control	MOE	EB	Movem	ent	WB	Moven	nent	NB	Movem	ent	SB Movement		nent	العدودي
Intersection	Period	Control	MOE	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Overall
			LOS		n/a 1					Α	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	Α
	AM		v/c		n/a <sup>1</sup>					0.0	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	
Hwy 67 at		One-Way	Delay		n/a <sup>1</sup>					7.4	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	0.0
Siplast Rd.		Stop	LOS		В					Α	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	Α
	PM		v/c		0.0					0.0	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	
			Delay		10.9					7.7	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	0.2
			LOS		В					Α	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	Α
	AM		v/c		0.3					0.0	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	
Hwy 67 at		One-Way Stop	Delay		13.7					7.5	n/a <sup>2</sup>			n/a²	n/a <sup>2</sup>	4.9
W Walnut St.			LOS		С					Α	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	Α
	PM		v/c		0.4					0.1	n/a <sup>2</sup>			n/a²	n/a <sup>2</sup>	
			Delay		16.0					7.7	n/a <sup>2</sup>			n/a²	n/a <sup>2</sup>	4.7
			LOS	n/a <sup>2</sup>	n/a <sup>2</sup>			E					Α		n/a <sup>2</sup>	В
	AM		v/c	n/a <sup>2</sup>	n/a <sup>2</sup>			0					0.0		n/a <sup>2</sup>	
Hwy 67/ Walnut St.		One-Way	Delay	n/a <sup>2</sup>	n/a <sup>2</sup>			11	1.7				8.0		n/a <sup>2</sup>	11.7
at Hwy 67 / 6th St.		Stop	LOS	n/a <sup>2</sup>	n/a <sup>2</sup>			E					Α		n/a <sup>2</sup>	Α
	PM		v/c	n/a <sup>2</sup>	n/a <sup>2</sup>			0					0.0		n/a <sup>2</sup>	
			Delay	n/a <sup>2</sup>	n/a <sup>2</sup>				3.1				7.7		n/a <sup>2</sup>	1.8
			LOS	Α	n/a <sup>2</sup>	n/a <sup>2</sup>	Α	n/a <sup>2</sup>	n/a <sup>2</sup>		С			С		Α
	AM		v/c	0.0	n/a <sup>2</sup>	n/a <sup>2</sup>	0.0	n/a ²	n/a²		0.3			0.1		
Hwy 67 / 6th St. at		Two-Way	Delay	7.8	n/a <sup>2</sup>	n/a <sup>2</sup>	8.0	n/a²	n/a <sup>2</sup>		17.3			16.6		4.1
Hwy 67 / 6th St. at Hwy 67 / Caddo St.		Stop	LOS	Α	n/a <sup>2</sup>	n/a <sup>2</sup>	Α	n/a <sup>2</sup>	n/a²		D			С		Α
	PM		v/c	0.0	n/a <sup>2</sup>	n/a <sup>2</sup>	0.0	n/a <sup>2</sup>	n/a²		0.6			0.3		
			Delay	7.9	n/a <sup>2</sup>	n/a <sup>2</sup>	8.2	n/a <sup>2</sup>	n/a <sup>2</sup>		33.8			20.1		8.7
			LOS		n/a <sup>2</sup>	n/a <sup>2</sup>	Α	n/a <sup>2</sup>			В					Α
	AM		v/c		n/a <sup>2</sup>	n/a <sup>2</sup>	0.0	n/a <sup>2</sup>			0.1					
Caddo St. at		One-Way	Delay		n/a <sup>2</sup>	n/a <sup>2</sup>	7.7	n/a <sup>2</sup>			11.9					1.0
S 3rd St.		Stop	LOS		n/a <sup>2</sup>	n/a <sup>2</sup>	Α	n/a <sup>2</sup>			В					Α
	PM		v/c		n/a <sup>2</sup>	n/a <sup>2</sup>	0.0	n/a <sup>2</sup>			0.2					
			Delay		n/a <sup>2</sup>	n/a <sup>2</sup>	8.0	n/a <sup>2</sup>			13.9					1.6
			LOS	В	Α	Α	С	Α	Α	Α	Α	Α	Α	Α	Α	В
	AM		v/c	0.3	0.0	0.0	0.6	0.0	0.0	0.1	0.0	0.0	0.4	0.0	0.0	
Hwy 67/ Caddo St.		Signal	Delay	17.4	0.0	0.0	20.3	0.0	0.0	4.0	0.0	0.0	5.8	0.0	0.0	12.1
at 10th St.		Olgridi	LOS	В	Α	Α	С	Α	Α	Α	Α	Α	Α	Α	Α	В
	PM		v/c	0.3	0.0	0.0	0.7	0.0	0.0	0.1	0.0	0.0	0.5	0.0	0.0	
			Delay	13.7	0.0	0.0	20.2	0.0	0.0	6.7	0.0	0.0	9.6	0.0	0.0	13.8
			LOS	С	Α	В	n/a 1	Α	В	Α	Α	Α	Α	Α	Α	В
	AM		v/c	0.3	0.0	0.6	n/a 1	0.0	0.4	0.1	0.0	0.2	0.4	0.0	0.0	
10th St. at Pine St.		Signal	Delay	20.9	0.0	19.3	n/a 1	0.0	18.2	0.1	0.0	0.3	6.2	0.0	0.0	10.2
TOUT OI. AL FINE OL.		Signal	LOS	В	Α	С	С	Α	Α	Α	Α	Α	Α	Α	Α	В
	PM		v/c	0.4	0.0	0.7	0.6	0.0	0.0	0.1	0.0	0.2	0.4	0.0	0.0	
		D							0.0		0.0		6.0	0.0		10.4

<sup>&</sup>lt;sup>1</sup>No volume modeled making this movement <sup>2</sup>Free movement <sup>3</sup>HCM methodology does not calculate LOS due to unconventional configuration

Table 9 (continued): Intersection Analysis – 2018 Existing Conditions – HCM

Intersection	Time Period Control		MOE	EB	Moven	nent	WB	Mover	nent	NB	Movem	ent	SB	Moven	nent	Overall
Intersection	Period	Control		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Overall
			LOS	n/a 1	Α	В	n/a 1	Α	В	Α	Α	Α	n/a 1	Α	Α	В
	AM		v/c	n/a 1	0.0	8.0	n/a 1	0.0	0.6	0.2	0.0	0.0	n/a 1	0.0	0.1	
12th St. at Pine St.		Signal	Delay	n/a 1	0.0	16.0	n/a 1	0.0	13.5	6.5	0.0	0.0	n/a 1	0.0	5.7	12.3
1201 01. 01. 110 01.		Olgridi	LOS	В	Α	Α	В	Α	Α	Α	Α	Α	n/a 1	Α	Α	В
	PM		v/c	0.6	0.0	0.0	0.7	0.0	0.0	0.1	0.0	0.0	n/a 1	0.0	0.2	
			Delay	12.3	0.0	0.0	13.3	0.0	0.0	6.5	0.0	0.0	n/a <sup>1</sup>	0.0	7.0	11.6
			LOS	Α	В	Α	Α	Α	В	В	Α	В	В	Α	В	В
	AM		v/c	0.1	0.5	0.0	0.1	0.0	0.4	0.2	0.0	0.1	0.3	0.0	0.2	
26th St. at Pine St.		Signal	Delay	9.0	14.6	9.6	9.4	0.0	13.5	16.0	0.0	15.5	17.6	0.0	16.8	14.7
ZOUT St. at FINE St.		Signal	LOS	В	В	Α	Α	Α	С	В	Α	В	В	Α	В	В
	PM		v/c	0.3	0.4	0.0	0.1	0.0	0.7	0.2	0.0	0.1	0.3	0.0	0.3	
			Delay	11.0	13.6	10.0	9.8	0.0	20.6	16.5	0.0	15.7	17.5	0.0	17.3	16.7
			LOS				С		С		Α		Α	Α		В
	AM		v/c				0.8		0.0		0.2		0.0	0.2		
Caddo St. at Pine		0:	Delay				30.9		22.2		9.3		4.8	3.0		10.8
St.		Signal	LOS				С		В		В		Α	Α		В
	PM		v/c				0.8		0.1		0.3		0.0	0.3		
			Delay				27.4		18.8		11.4		6.8	5.1		12.4
			LOS	В	В	В	В	В	В	В	Α	Α	n/a 1	n/a 1	n/a 1	В
	AM		v/c	0.0	0.6	0.6	0.0	0.4	0.4	0.3	0.0	0.0	n/a 1	n/a 1	n/a 1	
Walnut St. at Pine			Delay	12.7	15.9	16.0	17.2	12.0	11.9	14.6	0.0	0.0	n/a 1	n/a 1	n/a 1	14.4
St.		Signal	LOS	В	В	В	В	В	n/a 1	В	Α	Α	В	Α	Α	В
	PM		v/c	0.0	0.6	0.6	0.0	0.6	n/a 1	0.4	0.0	0.0	0.1	0.0	0.0	
			Delay	17.6	16.7	16.9	17.8	14.5	n/a 1	14.2	0.0	0.0	10.8	0.0	0.0	15.4
			LOS		n/a <sup>2</sup>	n/a <sup>2</sup>	Α	n/a 1	.,	С		В				Α
	AM		v/c		n/a <sup>2</sup>	n/a <sup>2</sup>	0.0	n/a 1		0.0		0.0				
Redhill Rd. at Pine		One-Way	Delay		n/a <sup>2</sup>	n/a <sup>2</sup>	9.9	n/a 1		18.7		11.5				0.1
St.		Stop	LOS		n/a <sup>2</sup>	n/a <sup>2</sup>	Α	n/a 1		С		В				Α
	PM		v/c		n/a <sup>2</sup>	n/a <sup>2</sup>	0.0	n/a 1		0.0		0.0				
			Delay		n/a <sup>2</sup>	n/a <sup>2</sup>	9.7	n/a 1		19.5		11.3				0.1
			LOS	В	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a²				D		В	Α
	AM		v/c	0.2	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>				0.1		0.1	
Professional Dr. at		One-Way	Delay	10.6	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>				31.7		11.3	1.5
Pine St.		Stop	LOS	В	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>				D		С	Α
	PM		v/c	0.1	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>				0.2		0.4	
			Delay	11.3	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>				29.3		17.0	2.3
			LOS	Α	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	F		В				Α
	AM		v/c	0.1	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	0.5		0.3				
I-30 NB Off Ramp at		One-Way	Delay	9.3	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	69.0		14.4				3.8
Pine St.		Stop	LOS	В	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	F		В				В
	PM		v/c	0.3	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	1.3		0.2				
			Delay	12.4	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	320.7		12.2				12.5
<sup>1</sup> No volume modela					II/a			II/a	11/a	U_U.,						0

<sup>&</sup>lt;sup>1</sup>No volume modeled making this movement <sup>2</sup>Free movement <sup>3</sup>HCM methodology does not calculate LOS due to unconventional configuration

Table 10: Intersection Analysis – 2018 Existing Conditions – SimTraffic

Intersection	Time	Control	MOE		Moven			Moven			Moven			Movem	nent	Overall
Increction	Period	CONTROL		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
	AM		LOS	n/a 1		n/a 1					4				4	Α
Hwy 67 at		One-Way	Delay	n/a 1		n/a 1					.0			2		1.3
Siplast Rd.	PM	Stop	LOS	Α		Α					4				4	Α
			Delay	4.3		1.0				0.	.9			2	.2	1.6
	AM		LOS	Α		Α				Α	Α			Α	Α	Α
Hwy 67 at	7 1111	One-Way	Delay	7.2		2.6				3.7	2.3			2.0	2.2	3.6
W Walnut St.	PM	Stop	LOS	Α		Α				Α	Α			Α	Α	Α
	1 141		Delay	7.4		2.4				3.6	2.0			2.3	2.2	3.2
	AM		LOS	Α	Α			Α	Α				Α		Α	Α
Hwy 67/ Walnut St.	VIAI	One-Way	Delay	1.1	1.4			7.2	2.5				1.9		1.8	1.8
at Hwy 67 / 6th St.	PM	Stop	LOS	Α	Α			Α	Α				Α		Α	Α
	L IAI		Delay	1.0	0.8			7.4	3.6				2.7		1.7	2.0
	AM		LOS	Α	Α	Α	Α	Α	Α	Α	Α	Α	Α	Α	n/a 1	Α
Hwy 67 / 6th St. at	AIVI	Two-Way	Delay	4.9	2.4	1.9	4.2	1.4	8.0	7.5	3.5	5.0	6.0	8.6	n/a 1	3.0
Hwy 67 / Caddo St.	PM	Stop	LOS	Α	Α	Α	Α	Α	Α	В	В	Α	Α	В	В	Α
	PIVI		Delay	3.3	2.6	1.8	4.4	2.3	1.3	11.0	12.9	8.1	9.5	11.0	14.7	4.6
	A N A		LOS		Α	Α	Α	Α		Α		Α				Α
Caddo St. at	AM	One-Way	Delay		1.2	0.8	4.4	1.7		5.6		3.8				1.7
S 3rd St.	DM	Stop	LOS		Α	Α	Α	Α		Α		Α				Α
	PM		Delay		1.2	0.7	2.0	2.5		6.3		4.0				2.3
			LOS	С	С	Α	n/a <sup>2</sup>	С	Α	n/a <sup>2</sup>	Α	Α	Α	Α	Α	В
Hwy 67/ Caddo St.	AM	0: 1	Delay	23.5	21.9	7.6	n/a <sup>2</sup>	21.7	9.3	n/a <sup>2</sup>	3.5	3.3	6.6	6.3	2.3	10.8
at 10th St.		Signal	LOS	С	В	В	С	С	В	В	Α	Α	Α	Α	Α	В
	PM		Delay	27.8	20.0	13.5	34.3	21.4	11.1	10.1	4.6	3.5	7.1	9.0	6.7	12.4
			LOS	С	С	В	n/a 1	В	Α	В	Α	Α	Α	Α	Α	В
1011 01 1 101	AM		Delay	29.3	27.9	14.4	n/a 1	17.5	6.5	11.2	3.1	5.2	7.9	6.7	4.1	11.4
10th St. at Pine St.		Signal	LOS	С	В	В	В	Α	В	Α	Α	n/a <sup>2</sup>	Α	Α	Α	В
	PM		Delay	32.4	19.7	11.4	14.1	18.4	8.8	12.6	4.0	n/a <sup>2</sup>	6.2	8.5	5.5	11.8
			LOS	n/a 1	В	Α	n/a 1	В	Α	В	Α	Α	n/a <sup>1</sup>	Α	Α	В
	AM		Delay	n/a 1	11.8	9.8	n/a 1	12.8	8.5	13.4	6.6	3.1	n/a 1	5.3	3.6	10.2
12th St. at Pine St.		Signal	LOS	В	Α	Α	В	В	Α	Α	Α	Α	n/a 1	Α	Α	Α
	PM		Delay	12.2	9.4	5.4	18.5	11.4	6.6	1.6	6.4	0.0	n/a 1	7.5	3.4	9.5
			LOS	Α	Α	Α	В	В	Α	В	В	Α	В	В	Α	Α
	AM		Delay	5.7	4.7	0.6	12.5	11.4	4.4	15.0	14.8	4.2	16.7	18.0	4.2	8.9
26th St. at Pine St.	Signal	LOS	В	A	A	В	В	В	В	В	A	В	В	A	В	
	PM		Delay	11.0	5.1	0.9	17.1	17.5	13.7	17.3	15.2	4.3	18.1	16.6	4.0	11.8
			LOS				С		A		A		A	A		A
Caddo St. at Pine	AM		Delay				26.4		3.6		0.8		5.0	5.9		6.9
St.		Signal	LOS				C		Α		Α		В	Α		Α
- 5.	PM	Delay				23.4		3.5		1.5		13.4	8.0		9.6	
			Delay				25.4		5.5		1.5		13.4	0.0		5.0

<sup>&</sup>lt;sup>1</sup>No Volume modeled making this movement

<sup>&</sup>lt;sup>2</sup>Low volume modeled making this movement

Table 10 (Continued): Intersection Analysis – 2018 Existing – SimTraffic

luto vo o oti o v	Time	Control	MOE	EB	Movem	nent	WB	Moven	nent	NB	Movem	ent	SB	Movem	nent	Overell
Intersection	Period	Control	MOE	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Overall
	AM		LOS	n/a <sup>2</sup>	В	Α	С	В	В	В	n/a <sup>2</sup>	Α	n/a <sup>1</sup>	n/a 1	n/a 1	В
Walnut St. at Pine	AIVI	Signal	Delay	n/a <sup>2</sup>	11.2	8.0	21.9	11.4	10.1	12.7	n/a <sup>2</sup>	4.6	n/a <sup>1</sup>	n/a 1	n/a 1	11.0
St.	PM	Signal	LOS	С	В	Α	С	В	n/a 1	Α	Α	Α	Α	Α	Α	В
	FIVI		Delay	22.3	13.3	9.3	21.1	13.4	n/a 1	13.8	0.9	9.9	6.1	0.8	7.1	12.8
	AM		LOS		Α	Α	Α	Α		С		Α				Α
Redhill Rd. at Pine	AIVI	One-Way	Delay		0.2	0.0	8.4	3.7		17.5		0.0				1.9
St.	PM	Stop	LOS		Α	Α	Α	Α		С		Α				Α
	LINI		Delay		0.2	0.0	4.6	4.7		24.2		0.0				2.7
	AM		LOS	Α	Α			Α	Α				С		Α	Α
Professional Dr. at	AIVI	One-Way	Delay	6.6	1.0			8.0	0.2				17.6		6.6	1.7
Pine St.	PM	Stop	LOS	Α	Α			Α	Α				Е		В	Α
	PIVI		Delay	9.5	8.0			1.0	0.3				41.7		12.0	2.9
	Λ N A		LOS	Α	Α			Α	Α	С	n/a <sup>2</sup>	Α				Α
I-30 NB Off Ramp at	t AM One-Way Stop	Delay	4.7	0.8			1.4	1.6	19.9	n/a <sup>2</sup>	1.5				1.9	
Pine St.		LOS	Α	Α			Α	Α	F	n/a 1	Α				Α	
	PIVI		Delay	9.6	0.7			2.0	1.7	68.0	n/a 1	1.6				4.2

#### **2018 Roadway Segment Analysis, Existing Conditions**

Where applicable, the roadway segments within the study area were analyzed using the *HCS* two-lane highway module. To the north of Walnut Street, Highway 67 crosses through Arkadelphia's Central Business District (CBD) and does not function as a highway. The number of access points per mile exceeds the range for a two-lane highway, and the base free flow speed (BFFS) is below the range for a two-lane highway according to the *HCM* methodology. Due to the close spacing of intersections and nature of operations along the roadway segments to the north of Walnut Street, these segments were not analyzed separately from the intersection analysis.

To the south of Walnut Street, two segments along Highway 67 were identified as functioning like a two-lane highway and were delineated according to typical section and speed limit as follows:

- Segment 1-Highway 67, from south of Siplast Road to north of Siplast Road (where the speed limit drops from 50 mph to 45 mph). This segment was identified as a Class I Two-Lane Highway and has an access point density of 8 points per mile.
- Segment 2- Highway 67, from north of Siplast Road to south of Walnut Street (where the speed limit drops from 45 mph to 40 mph). This segment was identified as a Class III Two-Lane Highway and has an access point density of 40 points per mile.

Based on the volume data, a peak hour factor of 0.82 was utilized for the AM peak and a peak hour factor of 0.86 was utilized for the PM peak. The percentage of heavy vehicles was 5% for the AM peak and 4% for the PM peak. Level terrain was assumed for both segments. The base free flow speed was estimated to be 50 mph based on the posted speed limit plus 10 mph according to *HCM* methodology.

The results of this analysis are shown in **Table 11** below. The complete results from this analysis are provided in **Appendix C - Roadway Segment Operational Analysis Results**. These results demonstrate that the corridor currently operates at acceptable LOS throughout the study area. However, the portion of Highway 67 from south of Siplast Road to north of Siplast Road operates at only marginally acceptable LOS D in both directions during one or both of the peak periods.

Table 11: Roadway Segment Analysis – 2018 Existing Conditions – HCM

	Two-Lane AM						PM					
Segment	Direction	Hwy Class	LOS	ATS (mph	PTS F (%)	PFF S (%)	v/c	LOS	ATS (mph	PTS F (%)	PFF S (%)	v/c
Hwy 67- South of Siplast Rd. to North of Siplast Rd.	NB	Class I	С	45.1	48.0		0.14	D	` ' _	_ ` _	91.4	
Hwy 67 - North of Siplast Rd. to South of Walnut St.	IND	Class III	В	29.8	59.2	85.1	0.14	С	28.0	50.0	79.9	0.11
Hwy 67 - South of Walnut St. to North of Siplast Rd.	SB	Class III	В	30.2	25.9	86.2	0.06	В	29.7	51.3	84.8	0.12
Hwy 67 - North of Siplast Rd. to South of Siplast Rd.	SD	Class I	D	43.9	21.0	91.6	0.05	D	44.0	44.7	91.7	0.12

#### 2040 FUTURE NO ACTION CONDITIONS

For the 2040 No Action Conditions, no improvements were assumed. The intersections and the highway segments were analyzed using the same methodologies and assumptions as were used for the 2018 Existing conditions. Only the volumes changed between the 2018 Existing and the 2040 No Action conditions. The results are described in the following subsections.

#### **2040 Intersection Analysis, No Action Conditions**

The results from the 2040 No Action intersection analysis are shown in **Tables 12 and 13**. The complete results are provided in **Appendix B - Intersection Operational Analysis Results.** This analysis showed that most movements at the study intersections operate acceptable under 2040 No Action conditions.

Table 12: Intersection Analysis – 2040 No Action Conditions – HCM

	Timo			EB	Moven	nent	WB	Mover	nent	NB I	Moveme	ent	SB	Moven	nent	
Intersection	Time Period	Control	MOE	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Overall
			LOS		n/a <sup>1</sup>					Α	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	Α
	AM		v/c		n/a <sup>1</sup>					0.0	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	
Hwy 67 at		One-Way	Delay		n/a <sup>1</sup>					7.5	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	0.0
Siplast Rd.		Stop	LOS		В					Α	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	Α
	PM		v/c		0.0					0.0	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	
			Delay		11.6					7.8	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	0.2
			LOS		С					Α	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	Α
	AM		v/c		0.4					0.0	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	
Hwy 67 at		One-Way	Delay		16.2					7.5	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	5.7
W Walnut St.		Stop	LOS		С					Α	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	Α
	PM		v/c		0.5					0.1	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	
			Delay		21.1					7.8	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	5.9
			LOS	n/a <sup>2</sup>	n/a <sup>2</sup>				В				Α		n/a <sup>2</sup>	В
	AM		v/c	n/a <sup>2</sup>	n/a <sup>2</sup>			0	.1				0.0		n/a <sup>2</sup>	
Hwy 67/ Walnut St.		One-Way	Delay	n/a <sup>2</sup>	n/a <sup>2</sup>			12	2.7				8.0		n/a <sup>2</sup>	11.7
at Hwy 67 / 6th St.		Stop	LOS	n/a <sup>2</sup>	n/a <sup>2</sup>			I	В				Α		n/a <sup>2</sup>	Α
	PM		v/c	n/a <sup>2</sup>	n/a <sup>2</sup>			0	.2				0.0		n/a <sup>2</sup>	
			Delay	n/a <sup>2</sup>	n/a <sup>2</sup>				1.9				7.9		n/a <sup>2</sup>	1.8
			LOS	Α	n/a <sup>2</sup>	n/a <sup>2</sup>	Α	n/a <sup>2</sup>	n/a²		С			С		Α
	AM		v/c	0.0	n/a <sup>2</sup>	n/a <sup>2</sup>	0.1	n/a <sup>2</sup>	n/a²		0.4			0.1		
Hwy 67 / 6th St. at		Two-Way	Delay	7.9	n/a <sup>2</sup>	n/a <sup>2</sup>	8.2	n/a <sup>2</sup>	n/a <sup>2</sup>		22.8			20.0		5.1
Hwy 67 / Caddo St.		Stop	LOS	Α	n/a <sup>2</sup>	n/a <sup>2</sup>	Α	n/a <sup>2</sup>	n/a²		F			D		С
	PM		v/c	0.0	n/a <sup>2</sup>	n/a <sup>2</sup>	0.0	n/a <sup>2</sup>	n/a²		1.0			0.4		
			Delay	8.1	n/a <sup>2</sup>	n/a <sup>2</sup>	8.4	n/a <sup>2</sup>	n/a <sup>2</sup>		96.1			27.4		21.7
			LOS		n/a <sup>2</sup>	n/a <sup>2</sup>	Α	n/a <sup>2</sup>			В					Α
	AM		v/c		n/a <sup>2</sup>	n/a <sup>2</sup>	0.0	n/a <sup>2</sup>			0.1					
Caddo St. at		One-Way	Delay		n/a <sup>2</sup>	n/a <sup>2</sup>	7.8	n/a <sup>2</sup>			13.0					1.1
S 3rd St.		Stop	LOS		n/a <sup>2</sup>	n/a <sup>2</sup>	Α	n/a <sup>2</sup>			С					Α
	PM		v/c		n/a <sup>2</sup>	n/a <sup>2</sup>	0.0	n/a <sup>2</sup>			0.2					
			Delay		n/a <sup>2</sup>	n/a <sup>2</sup>	8.2	n/a <sup>2</sup>			16.2					1.9
			LOS	В	Α	Α	В	Α	Α	Α	Α	Α	Α	Α	Α	Α
07/6	AM		v/c	0.3	0.0	0.0	0.7	0.0	0.0	0.0	0.0	0.0	0.4	0.0	0.0	
Hwy 67/ Caddo St. at 10th St.		Signal	Delay	16.0	0.0	0.0	19.6	0.0	0.0	4.8	0.0	0.0	1.5	0.0	0.0	9.7
at 10th St.	PM		LOS	В	A	A	C	A	A	A	A	A	A	A	A	В
	PIVI		v/c Delay	0.3 12.5	0.0	0.0	0.8	0.0	0.0	0.2 8.0	0.0	0.0	0.6 5.9	0.0	0.0	13.2
			LOS	C	Α	0.0 C	n/a 1	Α	В	Α	Α	Α	3.9 A	Α	Α	B
	AM		v/c	0.4	0.0	0.6	n/a 1	0.0	0.4	0.7	0.0	0.2	0.5	0.0	0.0	
			Delay	20.4	0.0	18.3	n/a 1	0.0	17.2	0.2	0.0	0.4	8.1	0.0	0.0	10.7
10th St. at Pine St.		Signal	LOS	В	Α	В	В	Α	Α	A	Α	Α	Α	Α	Α	В
	PM		v/c	0.4	0.0	0.7	0.6	0.0	0.0	0.2	0.0	0.3	0.5	0.0	0.0	
			Delay	18.5	0.0	19.0	19.3	0.0	0.0	0.7	0.0	0.8	8.2	0.0	0.0	10.7
<sup>1</sup> No volume model																

<sup>&</sup>lt;sup>1</sup>No volume modeled making this movement <sup>2</sup>Free movement <sup>3</sup>HCM methodology does not calculate LOS due to unconventional configuration

Table 12 (Continued): Intersection Analysis – 2040 No Action Conditions – HCM

	Time			EB	Movem	nent	WB	Moven	nent	NB	Movem	ent	SB	Moven	ent	
Intersection	Period	Control	MOE	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Overall
			LOS	n/a 1	Α	В	n/a <sup>1</sup>	Α	В	Α	Α	Α	n/a 1	Α	Α	В
	AM		v/c	n/a 1	0.0	0.8	n/a 1	0.0	0.6	0.3	0.0	0.0	n/a 1	0.0	0.1	
12th St. at Pine St.		Signal	Delay	n/a 1	0.0	15.8	n/a 1	0.0	12.8	8.0	0.0	0.0	n/a 1	0.0	6.8	12.4
12th St. at Fille St.		Signal	LOS	В	Α	Α	В	Α	Α	Α	Α	Α	n/a 1	Α	Α	В
	PM		v/c	0.6	0.0	0.0	0.7	0.0	0.0	0.1	0.0	0.0	n/a 1	0.0	0.2	
			Delay	12.5	0.0	0.0	13.9	0.0	0.0	7.7	0.0	0.0	n/a 1	0.0	8.4	12.2
			LOS	Α	В	Α	Α	Α	В	С	Α	В	F	Α	В	С
	AM		v/c	0.1	0.6	0.0	0.1	0.0	0.5	0.4	0.0	0.1	1.0	0.0	0.3	
26th St. at Pine St.		Signal	Delay	9.4	16.7	9.8	10.0	0.0	15.0	22.2	0.0	15.7	88.3	0.0	17.3	26.3
	PM	J	LOS v/c	B 0.4	B 0.5	B 0.1	A 0.1	C 0.8	Α	0.5	Α	B 0.1	F 1.1	Α	B 0.3	С
	L IAI		Delay	12.8	15.0	10.2	10.0	28.3		25.6		16.0	103.7		18.1	30.4
			LOS	12.0	10.0	10.2	C	20.0	С	20.0	В	10.0	Α	Α	10.1	В
	AM		v/c				0.8		0.0		0.4		0.0	0.2		
Caddo St. at Pine			Delay				29.7		21.0		10.6		6.0	3.6		11.4
St.		Signal	LOS				C		В		В		A	A		В
	PM		v/c				0.9		0.1		0.3		0.0	0.3		
			Delay				26.5		17.1		13.1		8.7	6.4		13.3
			LOS	В	В	В	В	В	В	В	Α	Α	n/a 1	n/a 1	n/a 1	В
	AM		v/c	0.0	0.7	0.7	0.0	0.4	0.4	0.4	0.0	0.0	n/a 1	n/a 1	n/a 1	
Walnut St. at Pine			Delay	12.1	16.2	16.4	17.8	11.2	11.1	17.5	0.0	0.0	n/a 1	n/a 1	n/a 1	14.8
St.		Signal	LOS	В	В	В	В	В	n/a 1	В	A	A	В	A	A	В
	PM		v/c	0.0	0.7	0.7	0.0	0.7	n/a 1	0.5	0.0	0.0	0.7	0.0	0.0	
			Delay	19.0	18.1	18.3	19.1	15.1	n/a 1	16.5	0.0	0.0	11.6	0.0	0.0	16.6
			LOS	10.0	n/a <sup>2</sup>	n/a <sup>2</sup>	В	n/a <sup>2</sup>	II/a	C	0.0	В	11.0	0.0	0.0	Α
	AM		v/c		n/a <sup>2</sup>	n/a <sup>2</sup>	0.0	n/a <sup>2</sup>		0.0		0.0				7.1
Redhill Rd. at Pine		One-Way	Delay		n/a <sup>2</sup>	n/a <sup>2</sup>	10.7	n/a <sup>2</sup>		22.3		12.5				0.2
St.		Stop	LOS		n/a <sup>2</sup>	n/a <sup>2</sup>	В	n/a <sup>2</sup>		С		В				Α
	PM	·	v/c		n/a <sup>2</sup>	n/a <sup>2</sup>	0.0	n/a <sup>2</sup>		0.0		0.0				
			Delay		n/a <sup>2</sup>	n/a <sup>2</sup>	10.4	n/a <sup>2</sup>		23.3		12.2				0.2
			LOS	В	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>				F		В	Α
	AM		v/c	0.3	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>				0.2		0.1	
Professional Dr. at		One-Way			n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>				58.3		12.3	2.0
Pine St.		Stop	LOS	В	n/a <sup>2</sup>			n/a <sup>2</sup>					Е		С	Α
	PM		v/c	0.2	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>				0.3		0.5	
	PM	Delay	13.2	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>				44.0		23.4	3.1	
			LOS	В	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	F		С	1 1.0		20.1	A
	AM One-Way I	v/c	0.2	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	1.0		0.4				, ,	
I-30 NB Off Ramp at			10.0	n/a n/a <sup>2</sup>			n/a n/a <sup>2</sup>	n/a n/a <sup>2</sup>	209.3		17.6				8.2	
Pine St.		LOS	C	n/a n/a <sup>2</sup>			n/a n/a <sup>2</sup>	n/a n/a <sup>2</sup>	F		17.0 B				6.2 E	
1 1110 01.			0.4				n/a n/a <sup>2</sup>									
	PM		v/c		n/a <sup>2</sup>				n/a <sup>2</sup>	2.9		0.2				20.0
		Delay	15.4	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	1119.1		13.7				39.8	

<sup>&</sup>lt;sup>1</sup>No volume modeled making this movement <sup>2</sup>Free movement

<sup>&</sup>lt;sup>3</sup>HCM methodology does not calculate LOS due to unconventional configuration

Table 13: Intersection Analysis – 2040 No Action Conditions – SimTraffic

Hwy 67 of at Spinst Rd.   Hwy 67 of at Spinst Rd.   Hwy 67 of at Spinst Rd.   PM   Sing   LoS   A   A   A   A   A   A   A   A   A		Time			EB	Movem	nent	WB	Moven	nent	NE	Movem	ent	SB	Moven	nent	
Marting   Mart	Intersection		Control	MOE	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Overall
Mart		0.04		LOS	n/a <sup>1</sup>		n/a <sup>1</sup>				,	4			,	Ą	Α
Siplast Rd.   PM	Hwy 67 at	AM	One-Way	Delay	n/a 1						2	.9			3	.4	1.9
Hwy 67 at   Hwy	Siplast Rd.	DM	Stop	LOS	Α						1	4			1	Ą	Α
Hwy 67 at   W Walnut St.   PM		PIVI		Delay	7.7		1.0				1	.1			3	.0	2.1
Hwy 67 at Walnut St. Walnut St. at Hwy 67 / Ghr St. at Hwy 67 / Gh		Δ1/4		LOS	Α		Α				Α	Α			Α	Α	Α
PM	Hwy 67 at	AIVI	One-Way	Delay	9.1		4.5				3.7	3.1			1.9	1.9	4.6
Hwy 67 / Walnut St at Hwy 67 / 6fth St. at Hwy 67	W Walnut St.	DM	Stop	LOS	Α		Α				Α	Α			Α	Α	Α
Hwy 67   Walnut St. at Hwy 67   6th St. at Pine St.		L IAI		Delay	7.3		2.3				4.9	3.3			2.8	2.6	3.8
Hwy 67 / 6th St. at Pine St.		ΔM		LOS	Α	Α			Α	Α				Α		Α	Α
PM	Hwy 67/ Walnut St.	AIVI	One-Way	Delay	1.2	1.8			7.4	2.4				0.9		1.2	1.6
Part	at Hwy 67 / 6th St.	DM	Stop	LOS	Α	Α			Α	Α				Α		Α	Α
Hwy 67 / 6th St at Pine St		FIVI		Delay	1.0	1.1			9.0	5.1				1.9		2.0	2.5
Hwy 67 / 6th St. at Hwy 67 / Caddo St. at Hwy 67 / Caddo St. at Pine St. PM  Stop PM  Stop PM  Stop Delay 4.2 2.3 1.7 4.2 2.3 2.7 10.3 3.1 7.6 6.6 8.4 n/a¹ 3.1 Hwy 67 / Caddo St. at Lat Pine St. PM  Stop Delay 4.5 2.9 2.0 5.2 2.2 1.7 14.1 17.2 8.7 8.1 12.6 7.0 5.6 Na A A A A A A A A A A A A A A A A A A		ΛM		LOS	Α	Α	Α	Α	Α	Α	В	Α	Α	Α	Α	n/a 1	Α
PM	Hwy 67 / 6th St. at	AIVI	Two-Way	Delay	4.2	2.3	1.7	4.2	2.3	2.7	10.3	3.1	7.6	6.6	8.4	n/a 1	3.1
Caddo St at S 3rd St.	Hwy 67 / Caddo St.	DM	Stop	LOS	Α	Α	Α	Α	Α	Α	В	С	Α	Α	В	Α	Α
Caddo St. at   S 3rd St.   PM		FIVI		Delay	4.5	2.9	2.0	5.2	2.2	1.7	14.1	17.2	8.7	8.1	12.6	7.0	5.6
Caddo St. at S 3rd St.   PM   Stop   LOS   A		ΛM		LOS		Α	Α	Α	Α		Α		Α				Α
PM	Caddo St. at	AIVI	One-Way	Delay		1.0	0.3	5.0	2.9		5.3		2.8				2.3
Delay	S 3rd St.	DM	Stop	LOS		Α	Α	Α	Α		Α		Α				Α
Hwy 67/ Caddo St at Pine St		PIVI		Delay		1.5	1.3	2.9	2.4		7.4		4.6				2.4
Hwy 67/ Caddo St. at 10th St. PM  Signal PM		Λ.Μ.		LOS	В	В	Α		С	В	Α	Α	Α	Α	Α	Α	В
PM	Hwy 67/ Caddo St.	AIVI	Signal	Delay	12.3	17.2	3.9	n/a <sup>2</sup>	22.7	12.0	2.5	2.9	2.0	7.2	7.2	4.0	11.1
Delay   47.0   33.4   32.8   9.9   21.8   15.6   5.3   5.7   3.7   10.5   10.7   7.7   16.6	at 10th St.	DM	Signal	LOS	D	С	С		С	В	Α	Α	Α	В	В	Α	В
10th St at Pine St. PM PM Signal PM  AM Signal PM Signal		L IAI		Delay	47.0	33.4	32.8		21.8	15.6	5.3	5.7	3.7	10.5	10.7	7.7	16.6
10th St. at Pine St.  PM  Signal PM  Signal PM  Signal PM  Signal PM  AM  21.8 19.8 12.4 n/a¹ 15.9 5.1 11.5 4.8 4.8 4.3 17.4 8.0 5.8 10.5  LOS C C B A B A B A C A A A A A A B B A C B A B A		ΛM		LOS	С	В	В	n/a 1	В	Α	В	Α	Α	В	Α	Α	В
PM	10th St. at Dina St	AIVI	Signal	Delay	21.8	19.8	12.4	n/a 1	15.9	5.1	11.5	4.8	4.3	17.4	8.0	5.8	10.5
Delay 25.8 22.6 12.2 7.5 15.1 7.9 20.2 5.3 2.7 10.0 9.9 7.0 12.8  AM Signal PM Signal PM Signal Signal Signal Signal RA Signal PM Signal Signal Signal PM Signal PM Signal PM Signal Signal Signal PM Signal PM Signal Sign	TOUT St. at FINE St.	DM	Signal	LOS	С	С	В	Α	В	Α	С	Α	Α	Α	Α	Α	В
12th St. at Pine St.  PM  Signal PM  Delay n/a¹ 11.2 9.3 n/a¹ 10.2 6.7 n/a² 7.4 5.0 n/a¹ 6.7 3.5 9.5		L IAI		Delay	25.8	22.6	12.2	7.5	15.1	7.9		5.3	2.7	10.0	9.9	7.0	12.8
12th St. at Pine St.  PM  Signal PM  Delay n/a¹ 11.2 9.3 n/a¹ 10.2 6.7 n/a² 7.4 5.0 n/a¹ 6.7 3.5 9.5		ΔΜ		LOS	n/a 1	В	Α	n/a 1	В	Α	n/a²	Α	Α	n/a 1	Α	Α	Α
PM	12th St at Dine St	AIVI	Signal	Delay	n/a 1	11.2	9.3		10.2	6.7	n/a²	7.4	5.0	n/a 1	6.7	3.5	9.5
Delay 19.1 12.0 10.4 n/a² 11.5 11.4 5.8 5.8 4.3 n/a¹ 8.9 3.3 10.6  AM Signal PM Signal St. at Pine St.  Delay 19.1 12.0 10.4 n/a² 11.5 11.4 5.8 5.8 4.3 n/a¹ 8.9 3.3 10.6  AM Signal PM Si	12th St. at I me St.	DM	Signal	LOS	В	В	В		В	В	Α	Α	Α	n/a 1	Α	Α	В
26th St. at Pine St.  PM  Signal PM  Signal PM  Signal PM  Signal PM  Signal PM  Signal PM  Delay 13.2 8.2 2.3 18.2 13.0 8.8 11.8 11.5 5.0 20.1 18.6 3.7 10.9  Caddo St. at Pine St.  Caddo St. at Pine St.  Delay 13.7 8.7 1.9 24.5 20.9 16.8 18.9 15.5 4.4 21.6 19.4 4.7 14.8  Caddo St. at Pine St.  Delay Caddo St. at Pine St.  Caddo St. at Pine St.  Delay Caddo St. at Pine St.  Delay Caddo St. at Pine St.  Caddo St. at Pine St.  Delay Caddo St. at Pine St.  Caddo St. at Pine St.  Delay Caddo St. at Pine St.  Caddo St. at Pine St.  Caddo St. at Pine St.  Delay Caddo St. at Pine St.  Caddo St. at Pine		L IAI		Delay	19.1	12.0	10.4	n/a <sup>2</sup>	11.5	11.4	5.8	5.8	4.3	n/a 1	8.9	3.3	10.6
26th St. at Pine St.  PM  Signal PM  Signal PM  Signal Signal PM  Signal Signal PM  Signal PM  Signal Signa		AM	_														
Delay 13.7 8.7 1.9 24.5 20.9 16.8 18.9 15.5 4.4 21.6 19.4 4.7 14.8  Caddo St. at Pine St.  Delay Signal PM  Signal PM  Delay IS.7 8.7 1.9 24.5 20.9 16.8 18.9 15.5 4.4 21.6 19.4 4.7 14.8  C A A B A A A A A A A A A A A A A A A A	26th St. at Pine St.		Signal														
Caddo St. at Pine St.         AM PM         Signal Pom         LOS Delay LOS         C C A A A B A A B A A A A A A A A A A A		PM															
Caddo St. at Pine St.  Signal PM				-	13.7	0.7	1.9		20.9		10.9		7.4			4.7	
St. Signal LOS C A A B A A	Caddo St. at Pine	AM															
PM 200			Signal														
		PM		Delay				20.6		4.2		1.2		12.8	8.1		8.6

 $<sup>^1\</sup>mbox{No}$  Volume modeled making this movement  $^2\mbox{Low}$  volume modeled making this movement

Table 13 (Continued): Intersection Analysis – 2040 No Action – SimTraffic

	Time			EB	Movem	nent	WB	Moven	nent	NE	3 Movem	ent	SB	Movem	nent	
Intersection	Period	Control	MOE	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Overall
	AM		LOS	n/a <sup>2</sup>	В	Α	С	Α	Α	В	Α	В	n/a 1	n/a 1	n/a 1	В
Walnut St. at Pine	AIVI	Signal	Delay	n/a <sup>2</sup>	11.6	9.3	26.4	7.8	2.8	15.3	9.0	10.8	n/a 1	n/a 1	n/a 1	10.6
St.	PM	Signal	LOS	D	В	В	С	В	n/a 1	В	В	Α	В	Α	Α	В
	FIVI		Delay	40.2	11.7	10.7	29.9	10.7	n/a 1	14.7	14.9	9.7	15.6	2.1	5.0	11.6
	AM		LOS		Α	Α	Α	Α		С		Α				Α
Redhill Rd. at Pine	AIVI	One-Way	Delay		0.2	0.0	7.4	3.0		22.4		0.0				1.5
St.	PM	Stop	LOS		Α	Α	Α	Α		В		Α				Α
	PIVI		Delay		0.1	0.0	5.8	4.2		12.0		0.3				2.4
	AM		LOS	Α	Α			Α	Α				F		Α	Α
Professional Dr. at	Aivi	One-Way	Delay	9.4	1.6			0.8	0.2				56.7		7.5	2.6
Pine St.	PM	Stop	LOS	В	Α			Α	Α				F		С	Α
			Delay	13.9	1.3			1.1	0.3				68.2		22.6	4.5
	AM		LOS	Α	Α			Α	Α	F	F	Α				Α
I-30 NB Off Ramp at	7	One-Way	Delay	9.8	0.9			1.5	1.7	59.2	109.0	3.0				3.8
Pine St.	PM	Stop	LOS	С	Α			Α	Α	F	n/a <sup>1</sup>	F				С
	I IVI		Delay	21.3	0.9			2.4	1.6	390	n/a 1	134.0				23.5

<sup>&</sup>lt;sup>1</sup>No Volume modeled making this movement

<sup>&</sup>lt;sup>2</sup>Low volume modeled making this movement

#### **2040 Roadway Segment Analysis, No Action Conditions**

The results from the *HCS* analysis of the Highway 67 roadway segments which operate as a two-lane highway are shown in **Table 14**. For this analysis, all assumptions and inputs remained the same as the 2018 Existing conditions except the volumes were updated to reflect the anticipated growth for 2040. Complete results of this analysis are provided in **Appendix C - Roadway Segment Operational Analysis Results**. Based on this analysis, the performance along these portions of Highway 67 will continue to perform adequately during both peak periods in 2040. However, the southernmost segment of Highway 67 from south of Siplast Road to north of Siplast Road will operate at marginally acceptable LOS D conditions for both directions of travel during both peak periods.

Table 14: Roadway Segment Analysis – 2040 No Action Conditions – HCM

		Two-Lane			AM					PM		
Segment	Direction	Hwy Class	LOS	ATS (mph		PFF S (%)	v/c	LOS	ATS (mph	PTS F (%)	PFF S (%)	v/c
Hwy 67- South of Siplast Rd. to North of Siplast Rd.	NB	Class I	D	44.4	52.1			D	43.4		90.3	0.11
Hwy 67 - North of Siplast Rd. to South of Walnut St.	ND	Class III	С	29.1	63.7	83.3	0.17	С	27.6	53.9	79.0	0.13
Hwy 67 - South of Walnut St. to North of Siplast Rd.	SB	Class III	В	30.0	26.6	85.7	0.07	С	29.1	54.7	83.1	0.14
Hwy 67 - North of Siplast Rd. to South of Siplast Rd.	SD	Class I	D	43.5	24.1	90.7	0.07	D	43.1	48.8	89.9	0.14

#### **SAFETY ANALYSIS**

In order to evaluate safety performance, the historical crashes occurring within the study area were collected for the three most recent complete years of available data (2015-2017). Generally, crash patterns on this corridor are typical of State highways in Arkansas: in urban areas, where traffic volumes are high, the frequency of rear-end and angle accidents tends to increase due to proliferation of access points. In rural areas, there is a higher incidence of single-vehicle (run-off-road) crashes.

Crash rates for total crashes and KA crashes were calculated for contiguous segments with similar geometric, developmental, and other characteristics along 4 separate segments of Hwy. 51 and Hwy. 67 and

KA Crashes are defined as either fatal or serious injury crashes.

compared to the statewide averages for similar facilities. Crash rates were calculated as follows:

Crash Rate (R) = 
$$(C * 10^6)/(V*365*N*L)$$

- R = Roadway crash rate expressed as crashes per Million Vehicle-Miles (MVM) of travel
  - o KA crash rate is expressed as crashes per 100 MVM of travel, thus (C\*108)
- C = Total number of roadway crashes in the study period
- V = Traffic volumes using Average Annual Daily Traffic volumes
- N = Number of years of data
- L = Length of the roadway segment in miles

**Table 15** shows the three-year average crash rates compared to statewide averages. The average crash rates on Highway 51 are above the statewide average for similar facilities. However, The KA crash rate was lower than the statewide average along the entire project corridor. The average crash rate on Highway 67 is also higher than the statewide average for similar facilities.

Table 15: Annual Average Crash Rates (2015 – 2017)

					All Crashes			KA Crashes	6
Route	Log Miles	Weighted ADT <sup>1</sup>	Segment Length	Number of Crashes	Crash Rate (per 100 MVM) <sup>4</sup>	Statewide Average (per 100 MVM) <sup>4</sup>	Number of Crashes	Crash Rate (per 100 MVM)	Statewide Average (per 100 MVM) <sup>5</sup>
Highway 51 (Walnut Street to 26th Street)	29.06 to 29.99	13,000	0.93	60	4.53	3.98 <sup>3</sup>	2	0.15	7.88
Highway 51 (26th Street to Robey)	30.00 to 31.27	9,000	1.27	47	3.76	2.48 <sup>2</sup>	0	0	9.55
Highway 67 (Siplast Road to Pine Street)	14.415 to 15.795	6,700	1.38	27	2.67	2.48 <sup>2</sup>	0	0	9.55
Highway 51 (1st Street to 5th Street)	0.00 to 0.51	3,400	0.51	8	4.21	2.48 <sup>2</sup>	0	0	9.55

<sup>1 -</sup> Average daily traffic

**Figure 7** illustrates locations with a high crash frequency. Crashes occurred more frequently along Highway 51 from Walnut Street to Robey Street, which are the most congested intersections on the corridor with frequent stop-and-go conditions. When considering fatal (K) and serious injury (A) crashes, as shown in **Figure 8**, the highest concentration is located between Walnut Street and 26<sup>th</sup> Street — as only 2 KA crashes occur within these project limits over the 3 year time period. A closer examination revealed these 2 KA crashes occurred due to a northbound vehicle attempting to turn left at Mercy Way in the inside through lane (because a turn bay wasn't present) and getting rear-ended by another vehicle.

The safety analysis also revealed a relatively high number of crashes at the unsignalized intersections of Highway 51 between the I-30 northbound ramp and Professional Park

<sup>2 -</sup> Statewide average crash rate for two-lane undivided highways, no control of access

<sup>3 -</sup> Statewide average crash rate for four-lane undivided highways, no control of access

<sup>4 -</sup> Crash rates reported in crashes per million vehicle miles (MVM)

<sup>5 -</sup> KA crash rates reported in crashes per 100 million vehicle miles (MVM)

The safety analysis also revealed a relatively high number of crashes at the unsignalized intersections of Highway 51 between the I-30 northbound ramp and Professional Park Drive. Eastbound cars turning left are not protected resulting in angle crashes. Several rear-end crashes occur here potentially due to a westbound following driver incorrectly assuming that the drivers will be accelerating onto the interstate ramp.

At the signalized intersection at Pine Street and Walnut Street, a pattern of angle crashes occur, which could be due to the roadway curve through the intersection.

The roadway configuration at Pine Street and Caddo Street, coupled with the lack on signage, could play a role in the number of crashes occurring near this intersection. Vehicles eastbound on Street have the opportunity to turn right onto Caddo Street, approximately 150 feet upstream of the signal. There is no physical barrier preventing westbound vehicles using from turn left at this location. The confusion of this configuration may add to the number of angle crashes occurring here.

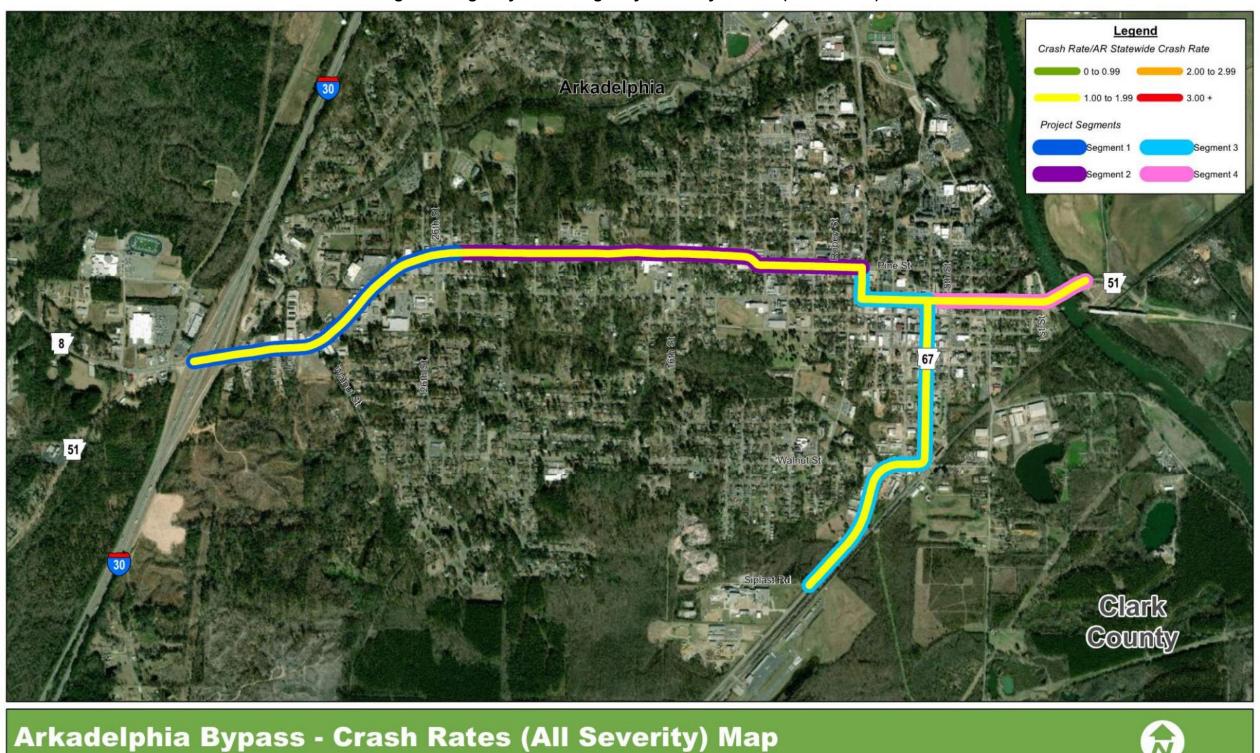


Figure 7: Highway 51 and Highway 67 Safety Review (All Crashes)

Arkadelphia Bypass Traffic Study

Arkadelphia Clark County

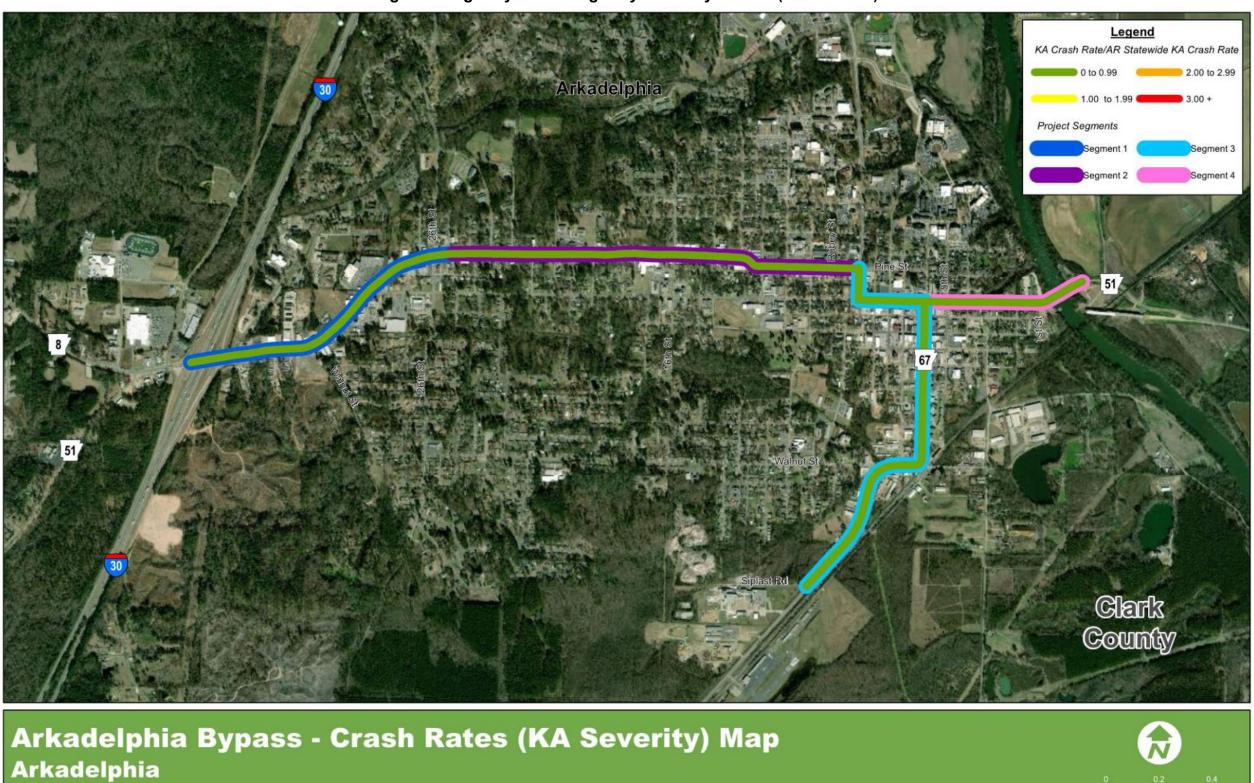


Figure 8: Highway 51 and Highway 67 Safety Review (KA Crashes)

Arkadelphia Bypass Traffic Study

**Clark County** 

# **ALTERNATIVES**

Based on an evaluation of the existing conditions as well as future transportation needs, various bypass corridors have been identified. The proposed bypass alignment generally intersects Highway 51 near the I-30 interchange, passes south of Arkadelphia, intersects Highway 67 and continues east, intersecting with Highway 51 on the east side of Arkadelphia. A modified bypass option that intersects I-30 at a new interchange south of the I-30 interchange at Highway 51 was also considered as part of the report. This alternative removes all bypass traffic from the intersections on Highway 51 / Pine Street. To evaluate the impacts of a future bypass, the study area was separated into western improvement alternatives and eastern improvement alternatives. For each alternative analysis, it was assumed the full bypass would be constructed by 2040.

## **2040 ALTERNATIVES VOLUMES**

To develop 2040 Alternative Volumes, the 2018 Existing Volumes were redistributed based on the Projected Bypass Volumes for Alternative 3 and Alternative 7 from the previous study. On Walnut Street near the Pine Street intersection, the previous study showed the bypass would carry an average daily traffic (ADT) volume of 5,200 in 2013. Along the bypass south of Arkadelphia, the report showed an ADT of 1,300 in 2013. These ADTs were projected to 2018 and AM and PM peak hour volumes were developed, assuming approximately 9.5% and 10.5% of the ADT respectively. The traffic volume expected to use the proposed bypass were removed from the traffic volumes currently using Highway 51 through the city.

Like the 2040 No Action volumes, 2040 Alternative volumes were projected from the 2018 redistributed volumes using the exponential growth rate of 0.80%. Future traffic volumes for western improvement alternative are shown in **Figures 9** through **Figure 13**.

Additional development will likely occur within this portion of study area but should not significantly increase the traffic volumes on the existing and proposed roadway network. Future volumes for the eastern improvement alternatives are shown in **Figure 14** and **Figure 15.** Due to very similar alignments, future volumes at key intersections for Bypass Alignment E and Bypass Alignment F are shown on Figure 14.

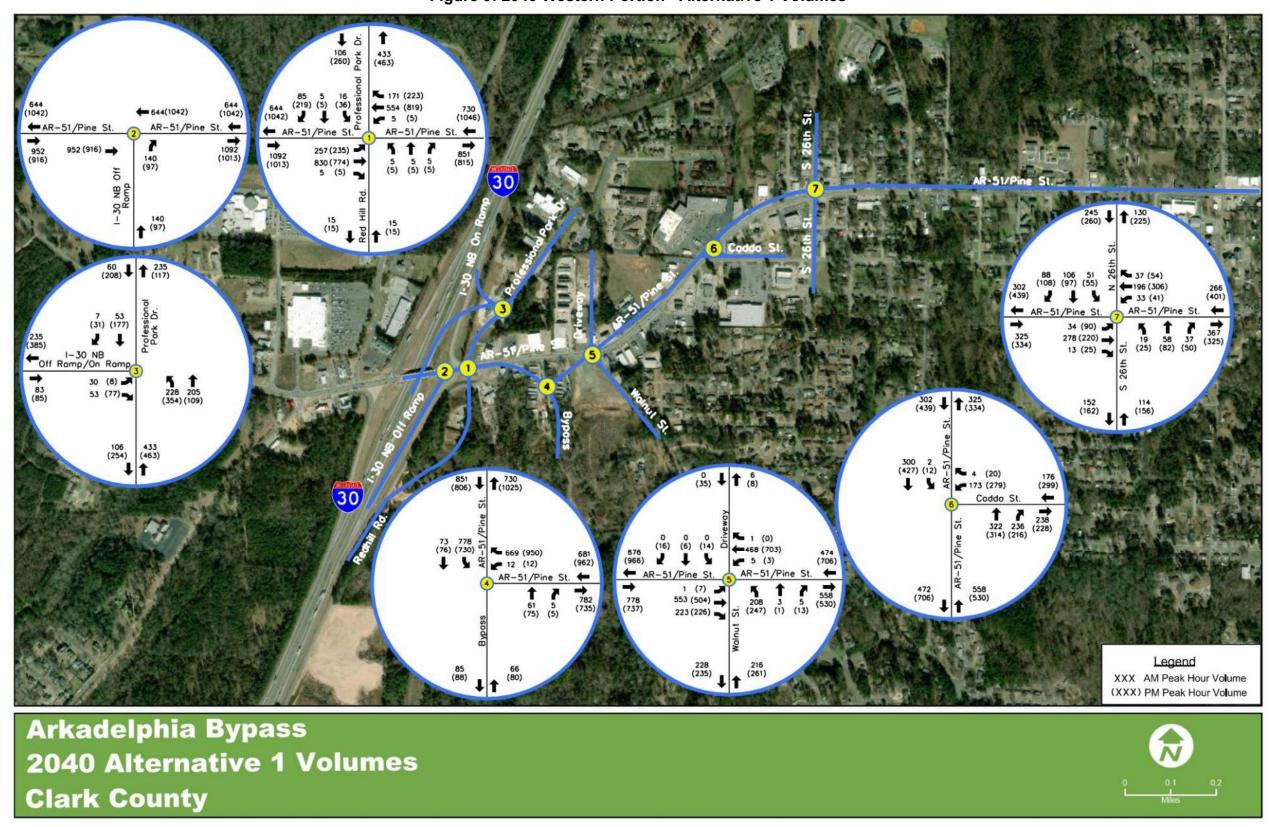


Figure 9: 2040 Western Portion - Alternative 1 Volumes

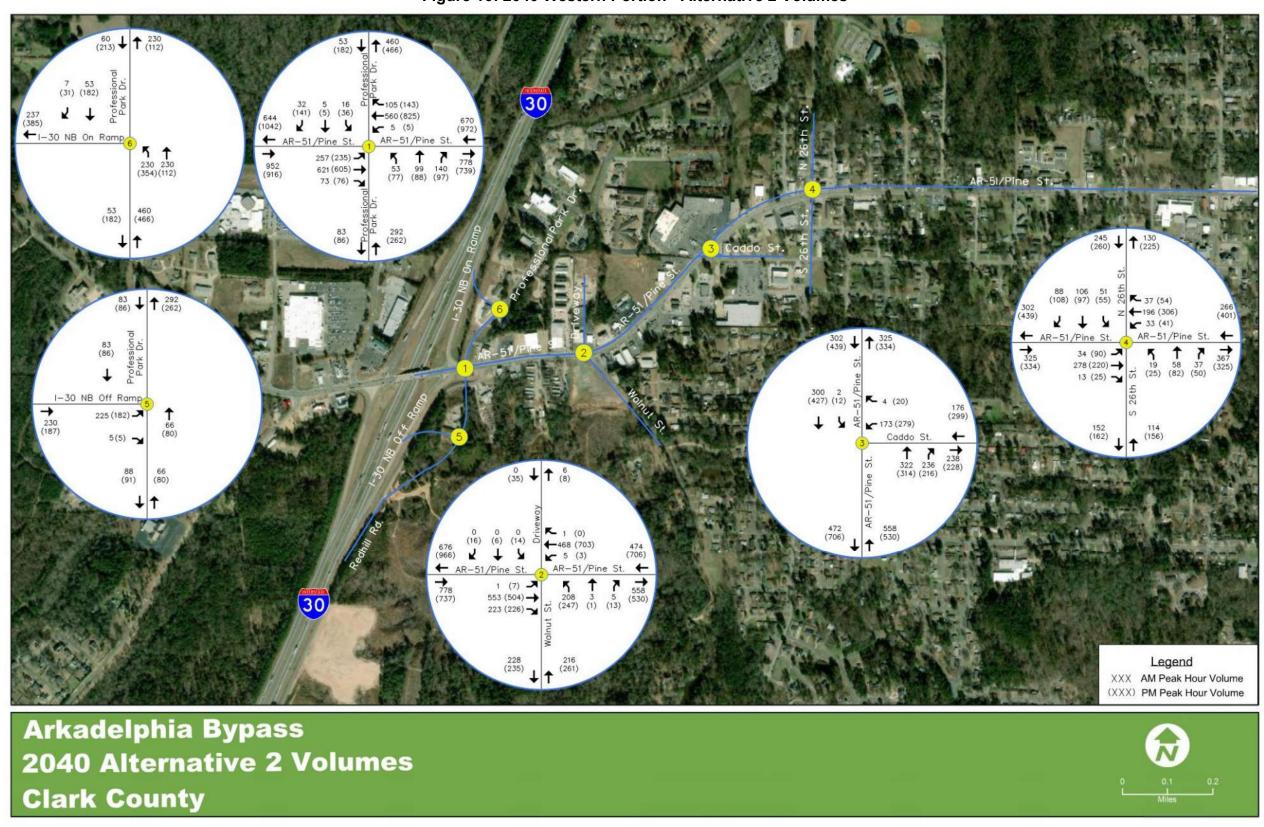


Figure 10: 2040 Western Portion - Alternative 2 Volumes

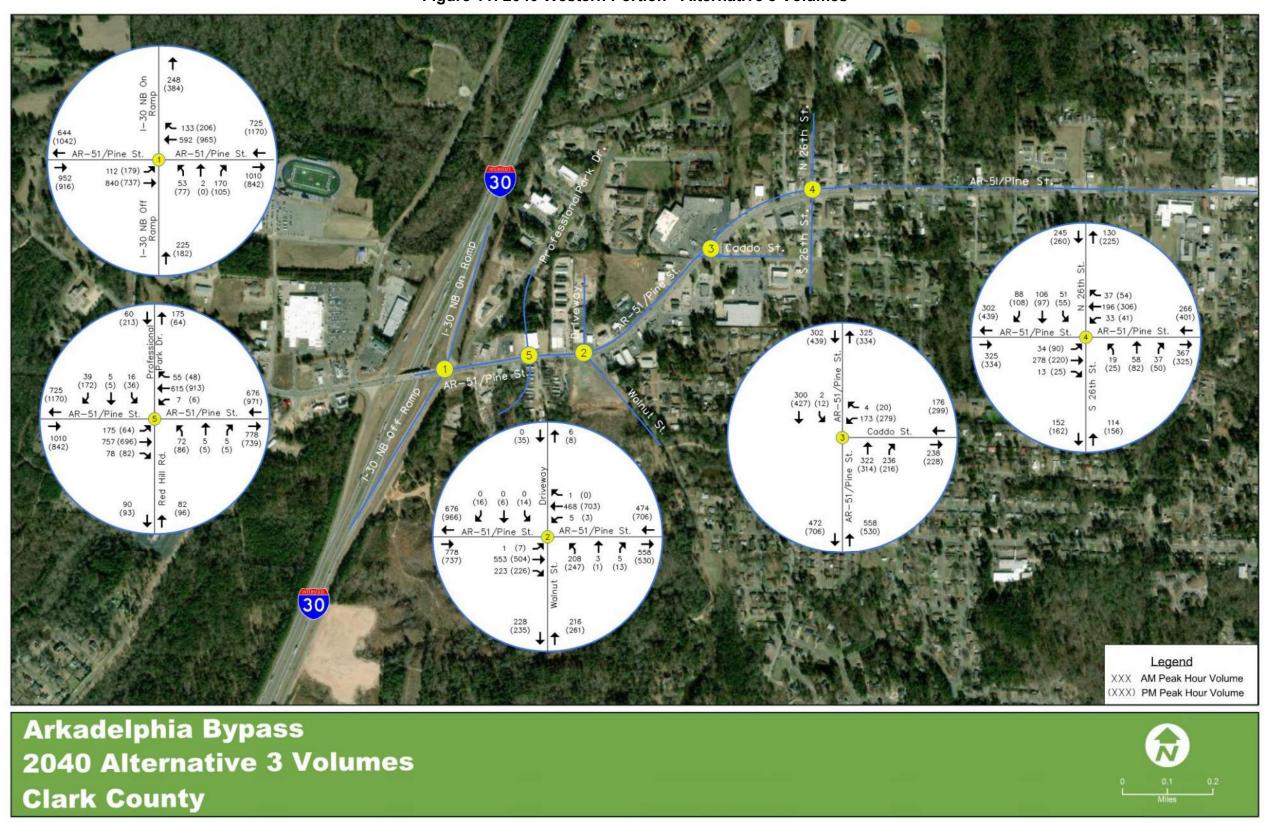


Figure 11: 2040 Western Portion - Alternative 3 Volumes

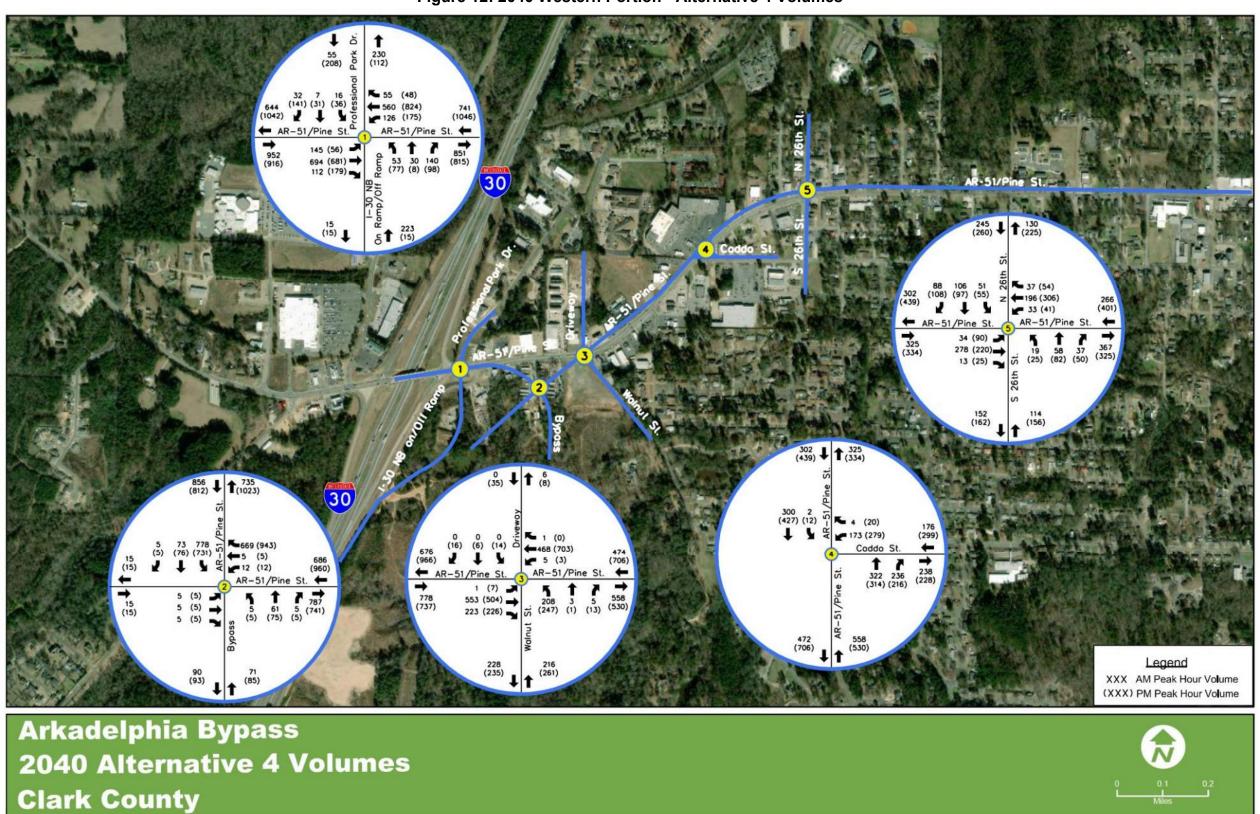


Figure 12: 2040 Western Portion - Alternative 4 Volumes

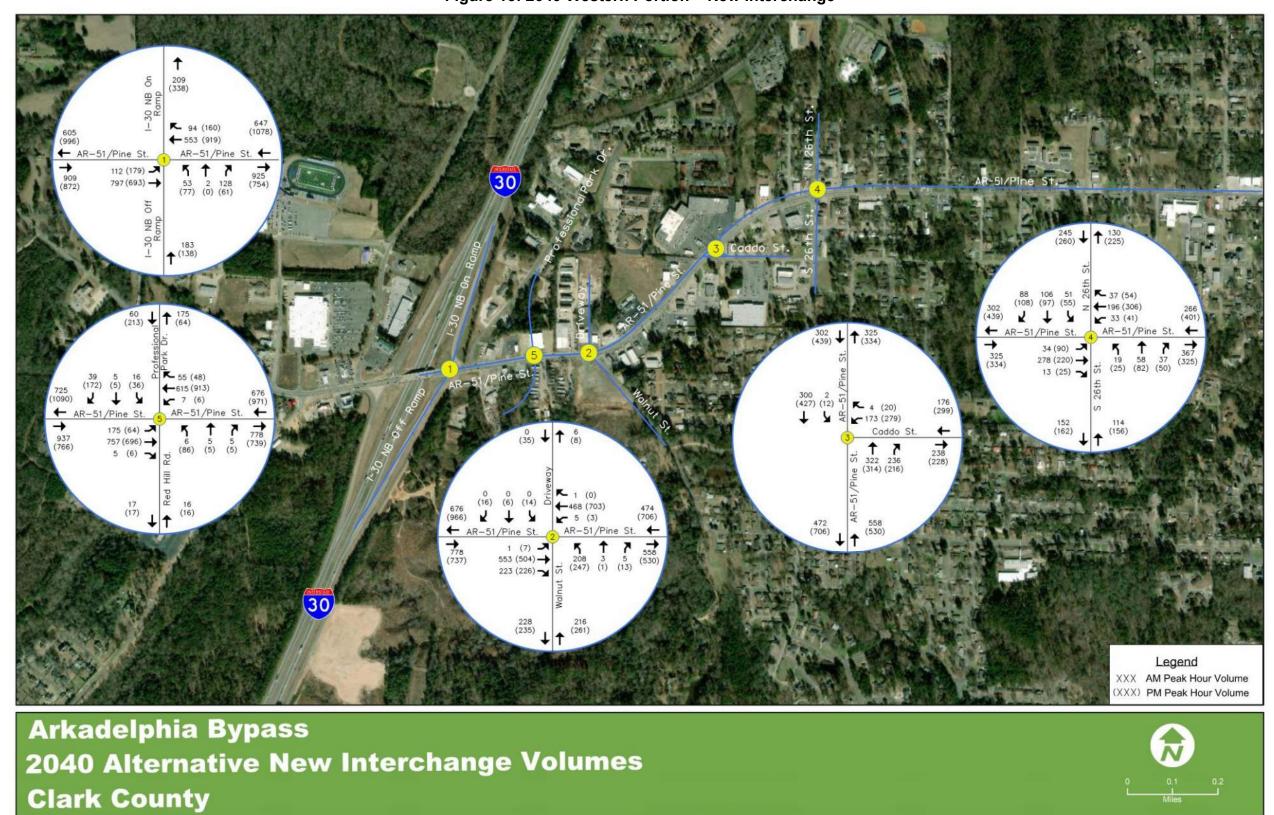


Figure 13: 2040 Western Portion – New Interchange

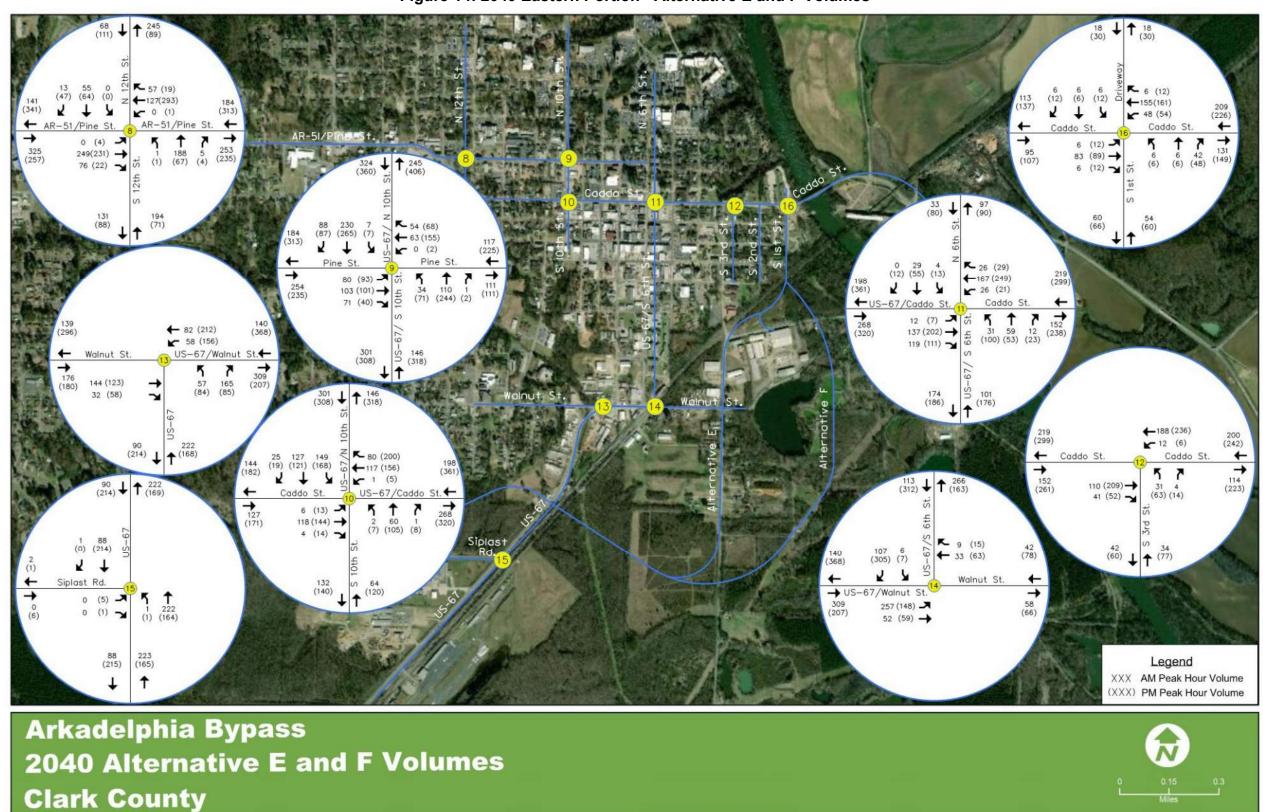


Figure 14: 2040 Eastern Portion - Alternative E and F Volumes

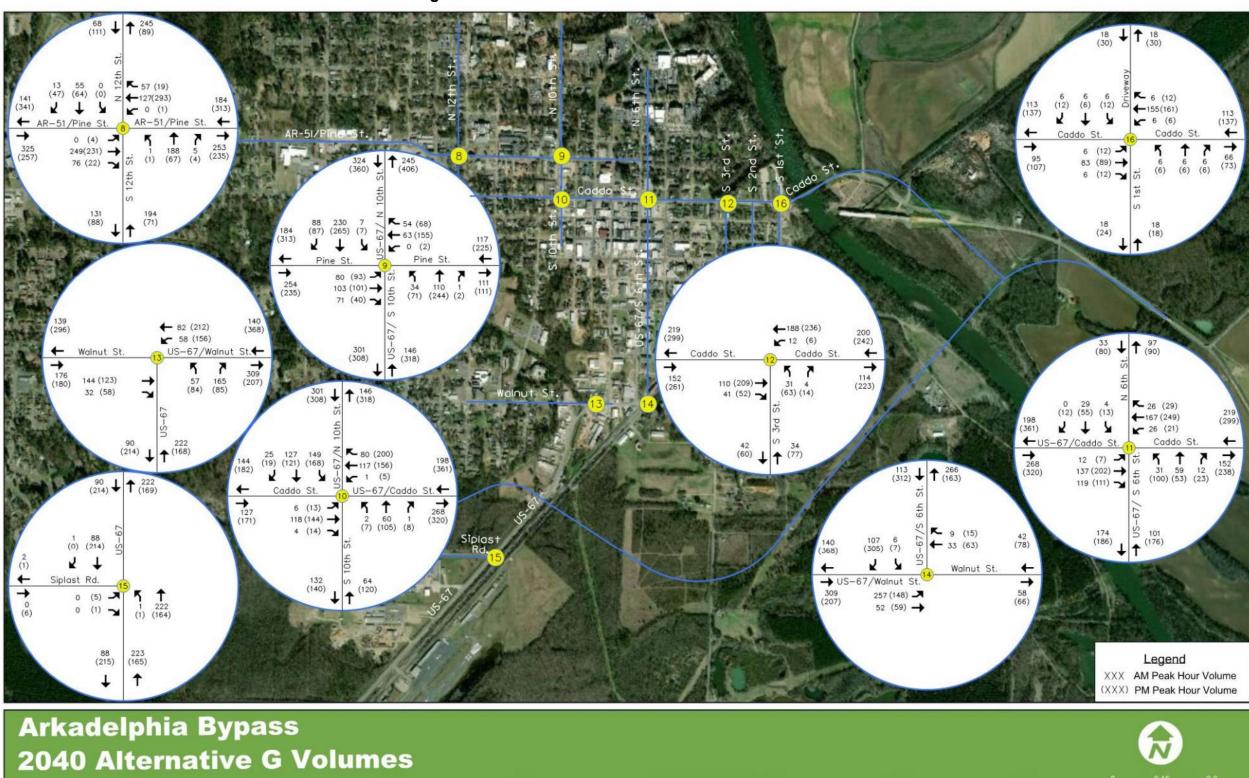


Figure 15: 2040 Eastern Portion - Alternative G Volumes

**Clark County** 

#### 2040 ALTERNATIVES ANALSYIS

Traffic operations were evaluated for all bypass alternatives. The intersections were analyzed using the same methodologies as the Existing and the 2040 No Action conditions. All Action Alternative conditions assumed completion of the full bypass for 2040 and were evaluated as western improvement alternatives and eastern improvement alternatives. For the western bypass alternatives that extended to the existing interchange, the improved intersections near the interchange were analyzed as signals and as roundabouts. For the new interchange alternative, the study intersections were analyzed only as signals. The results of the intersection analyses and the roadway segment analyses are described in the following subsections.

### **2040 Intersection Analysis, Western Alternative Conditions**

The results from the western intersection analyses for both signalized intersections and roundabout intersections are shown in **Table 16** through **Table 31**. The results from the analyses that included the new interchange are shown in **Table 32** and **Table 33**. The complete results are provided in **Appendix B - Intersection Operational Analysis Results.** 

The results of the operational analysis of the Western Action Alternatives indicate similar delays for each scenario. Generally, the overall intersection LOS for the signalized intersections on the western end of study area operate acceptably during AM and PM peak hours, with an unacceptable LOS for certain movements for Alternatives 1, 2 and 3. The western end of the study area operates well with roundabout intersections during both AM and PM peak hours for all proposed Action Alternatives. As would be expected with the new interchange alternative, removing bypass traffic from Highway 51/Pine Street between the interchange and Walnut decreases overall delay at each intersection during AM and PM peak hours.

Table 16: Intersection Analysis – 2040 Alternative 1 (Signals) – HCM

late are effect	Time - Denie d	Ozarlani	MOE	EB	Movem	nent	WE	8 Moven	nent	NE	Movem	ent	SB	Movem	ent	0
Intersection	Time Period	Control	MOE	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Overall
			LOS	Α	В	Α	Α	Α	В	С	Α	В	F	Α	В	С
	AM		v/c	0.1	0.5	0.0	0.1	0.0	0.4	0.4	0.0	0.1	1.0	0.0	0.3	
26th St. at Pine St.		Signal	Delay	9.0	14.1	9.8	9.2	0.0	13.4	22.2	0.0	15.7	88.3	0.0	17.3	26.8
20th St. at r me St.		Signal	LOS	В	В	В	Α	Α	В	С	Α	В	F	Α	В	С
	PM		v/c	0.3	0.4	0.1	0.1	0.0	0.7	0.5	0.0	0.1	1.1	0.0	0.3	
			Delay	10.9	13.2	10.3	9.7	0.0	19.9	25.5	0.0	16.0	103.6	0.0	18.0	28.6
			LOS				С		С		Α		Α	Α		Α
	AM		v/c				8.0		0.0		0.2		0.0	0.2		
Pine St. at Caddo St.		Signal	Delay				29.6		21.0		3.6		3.9	3.5		9.3
		0.9.14.	LOS				С		В		Α		Α	Α		В
	PM		v/c				0.9		0.1		0.2		0.0	0.3		
			Delay				26.2		17.1		5.8		6.5	6.1		11.2
			LOS	В	С	С	С	В	В	В	Α	Α	n/a 1	n/a 1	n/a 1	В
	AM		v/c	0.0	8.0	8.0	0.0	0.5	0.5	0.3	0.3	0.4	n/a 1	n/a 1	n/a 1	
Walnut St. at Pine St.		Signal	Delay	17.2	21.6	21.8	24.4	14.7	14.7	11.5	0.0	0.0	n/a 1	n/a 1	n/a 1	17.9
vaniut St. at Pille St.		3 -	LOS	С	С	С	С	В	n/a 1	В	Α	Α	Α	Α	Α	В
	PM		v/c	0.0	0.8	8.0	0.0	0.8	n/a 1	0.4	0.2	0.0	0.1	0.0	0.0	
			Delay	24.0	23.5	23.8	25.1	19.6	n/a 1	11.4	0.0	0.0	7.8	0.0	0.0	19.8
			LOS	D	В	В	E	В	В	С	С	С	С	С	С	В
Red Hill Rd. /	AM		v/c	0.8	0.5	0.5	0.5	0.4	0.3	0.0	0.0	0.0	0.1	0.0	0.2	
Professional Park Dr.		Signal	Delay	41.4	11.0	10.9	68.2	14.8	13.9	28.0	27.8	27.3	28.5	27.8	21.7	17.2
at Pine St.	D		LOS	D	В	В	D	С	В	В	В	В	В	В	В	С
	PM		v/c	0.8	0.6	0.6	0.5	0.9	0.5	0.0	0.0	0.0	0.8	0.0	0.5	0.1.0
			Delay	41.8	15.9	15.8	49.7	25.2	18.2	14.9	14.8	14.5	15.6	14.8	15.1	21.9
	AM		LOS	D		D				D	Α			В	В	С
5 ( ) 15 15	AIVI		v/c	0.4		0.7				0.8	0.1			0.1	0.0	22.0
Professional Park Dr. at I-30 NB Ramps		Signal	Delay	42.4 D		50.8 D				37.8	0.1 A			12.7 B	12.2 B	23.2 C
at 1-00 NB Namps	PM		v/c	0.1		0.8				D 0.8	0.1			0.3	0.1	C
	PIVI															20.2
			Delay	38.3		51.7	С		В	38.1	5.1 C	В	С	16.6 A	14.2	28.2 B
	AM		V/C				0.0		0.6		0.1	0.0	0.9	0.0		D
	Aivi		Delay				28.7		14.7		22.2	10.7	25.4	0.0		19.6
Bypass at Pine St.		Signal	LOS				20.7 C		14.7 B		C C	10.7 B	25.4 C	Ο.0		19.0 C
	PM		v/c				0.0		18.1		0.1	0.0	0.9	0.0		
	I IVI		Delay				26.3		18.3		24.0	10.3	33.9	5.4		24.1
			Delay				20.3		10.3		24.0	10.3	აა.ყ	5.4		24.1

<sup>&</sup>lt;sup>1</sup>No volume modeled making this movement

<sup>&</sup>lt;sup>2</sup>Free movement <sup>3</sup>HCM methodology does not calculate LOS due to unconventional configuration

Table 17: Intersection Analysis – 2040 Alternative 1 (Signals) – SimTraffic

Intersection	Time	Control	MOE	EB	Movem	ent	WE	Movem	nent	NB	Movem	ent	SB	Movem	ent	Overall
Intersection	Period	Control	IVIOE	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Overall
	AM		LOS	В	В	Α	В	Α	Α	В	В	Α	В	В	Α	В
26th St. at Pine St.	AIVI	Cianal	Delay	13.5	17.0	2.6	11.3	9.7	6.8	17.2	15.7	3.9	19.2	16.2	3.7	12.5
Zoth St. at Pine St.	PM	Signal	LOS	В	В	Α	В	С	В	В	В	Α	В	В	Α	В
	FIVI		Delay	16.4	16.1	3.9	18.4	22.0	17.6	18.0	13.0	3.9	17.2	18.7	3.8	16.3
	AM		LOS				С		Α		Α		С	Α		Α
Pine St. at Caddo St.	AIVI	Cianal	Delay				24.4		2.0		1.2		23.9	4.6		7.4
Fille St. at Caudo St.	PM	Signal	LOS				В		Α		Α		В	Α		Α
	L IAI		Delay				19.7		1.9		2.1		15.7	9.5		9.8
	AM		LOS	n/a <sup>2</sup>	С	С	F	В	n/a <sup>2</sup>	Α	В	Α	n/a 1	n/a 1	n/a 1	С
Walnut St. at Pine St.	AIVI	Signal	Delay	n/a <sup>2</sup>	29.2	28.5	91.4	17.9	n/a <sup>2</sup>	9.8	17.5	6.6	n/a <sup>1</sup>	n/a 1	n/a 1	22.5
Walliut St. at Fille St.	DM	Signal	LOS	n/a 1	В	В	D	В	n/a 1	В	n/a 1	Α	Α	В	Α	В
	PM		Delay	n/a 1	16.4	12.5	35.1	19.3	n/a 1	10.5	n/a 1	7.0	5.0	10.2	4.3	15.9
	AM		LOS	С	Α	Α	D	С	Α	С	С	С	D	С	Α	В
Red Hill Rd. / Professional Park Dr.	AIVI	Signal	Delay	33.9	8.5	4.3	44.3	22.6	8.3	34.1	28.4	21.3	44.8	31.2	3.7	16.2
at Pine St.	PM	Signal	LOS	С	В	Α	С	С	Α	Α	С	Α	В	С	Α	В
	FIVI		Delay	29.3	14.0	1.1	23.9	27.1	9.9	7.8	20.1	0.0	15.5	23.1	8.0	19.3
	AM		LOS	D		Α				Α	Α			С	Α	Α
Professional Park Dr.	Alvi	Signal	Delay	43.7		4.3				6.3	3.7			28.4	3.2	9.0
at I-30 NB Ramps	DM	Sigilal	LOS	Е		Α				В	Α			С	Α	В
	PM		Delay	56.7		5.3				10.8	1.7			28.0	5.8	12.7
	ΛM		LOS				Е		Α		D	Α	D	Α		С
Punges at Ding St	AM Bypass at Pine St.	One-Way	Delay				43.6		8.6		31.4	6.4	28.8	4.4		19.2
Dypass at Fille St.	PM	Stop	LOS				Е		В		D	Α	С	Α		С
	FIVI		Delay				39.7		10.3		30.2	6.7	22.1	5.4		15.5

<sup>&</sup>lt;sup>1</sup>No Volume modeled making this movement

<sup>&</sup>lt;sup>2</sup>Low volume modeled making this movement

Table 18: Intersection Analysis – 2040 Alternative 1 (Roundabouts) – HCM

Intersection	Time	Control	MOE	EB	Movem	ent	WE	3 Movem	nent	NE	Movem	ent	SE	3 Movem	ent	Overall
IIIIGISGCIIOII	Period	Control	IVIOL	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Overall
			LOS	Α	В	Α	Α	Α	В	С	Α	В	F	Α	В	С
	AM		v/c	0.1	0.5	0.0	0.1	0.0	0.4	0.4	0.0	0.1	1.0	0.0	0.3	
00th 0t at Dia - 0t		0:	Delay	9.0	14.1	9.8	9.2	0.0	13.4	22.2	0.0	15.7	88.3	0.0	17.3	26.8
26th St. at Pine St.		Signal	LOS	В	В	В	Α	Α	В	С	Α	В	F	Α	В	С
	PM		v/c	0.3	0.4	0.1	0.1	0.0	0.7	0.5	0.0	0.1	1.1	0.0	0.3	
			Delay	10.9	13.2	10.3	9.7	0.0	19.9	25.5	0.0	16.0	103.6	0.0	18.0	28.6
			LOS				С		С		Α		Α	Α		Α
	AM		v/c				0.8		0.0		0.2		0.0	0.2		
Pine St. at Caddo St.		Signal	Delay				29.6		21.0		3.6		3.9	3.5		9.3
i iiio ot. ut ouddo ot.		Oigilai	LOS				С		В		Α		Α	Α		В
	PM		v/c				0.9		0.1		0.2		0.0	0.3		
			Delay				26.2		17.4		5.8		6.5	6.1		11.7
			LOS	В	С	С	С	В	В	В	Α	Α	n/a 1	n/a 1	n/a 1	В
	AM		v/c	0.0	8.0	8.0	0.0	0.5	0.5	0.3	0.0	0.0	n/a 1	n/a 1	n/a 1	
Walnut St. at Pine St.		Signal	Delay	17.2	22.5	22.8	24.4	14.7	14.7	11.5	0.0	0.0	n/a 1	n/a 1	n/a 1	18.5
Walnut St. at Pine St.		Olgilai	LOS	С	С	С	С	В	n/a <sup>1</sup>	В	Α	Α	Α	Α	Α	С
	PM		v/c	0.1	8.0	0.8	0.0	8.0	n/a 1	0.4	0.2	0.4	0.1	0.0	0.0	
			Delay	24.1	24.7	25.1	25.1	19.6	n/a 1	11.4	0.0	0.0	7.8	0.0	0.0	20.4
			LOS		Α						Α			Α		Α
	AM		v/c		0.1						0.4			0.1		
Professional Park Dr.		Roundabout	Delay		3.6						6.9			4.3		6.2
at I-30 NB Ramps		rtoundabout	LOS		Α						Α			Α		Α
	PM		v/c		0.1						0.4			0.3		
			Delay		4.3						7.0			7.7		6.9
	A B 4		LOS		A			A			A			A		Α
Red Hill Rd. /	AM		v/c		0.5 8.4			0.5 9.0			0.0 9.2			0.2 6.8		0.5
Professional Park Dr.		Roundabout	Delay		0.4 A			9.0 B			9.2 A			0.0 C		8.5 B
at Pine St.	PM		v/c		0.5			0.6			0.0			0.6		В
	FIVI		Delay		8.0			12.9			8.6			18.0		11.3
			LOS		0.0			12.9 A			Α			10.0 B		B
	AM		v/c					0.3			0.1			0.8		
	7 (141		Delay					6.0			7.7			14.5		10.1
Bypass at Pine St.		Roundabout	LOS					Α			Α			В		Α
	PM		v/c					0.5			0.1			0.7		
			Delay					8.1			7.1			12.7		9.7

<sup>&</sup>lt;sup>1</sup>No volume modeled making this movement <sup>2</sup>Free movement

<sup>&</sup>lt;sup>3</sup>HCM methodology does not calculate LOS due to unconventional configuration

Table 19: Intersection Analysis – 2040 Alternative 1 (Roundabouts) – SIDRA

la ta una ati a u	Time	Cantral	МОЕ	EB	Movem	ent	WE	3 Movem	nent	NE	3 Movem	nent	SE	3 Movem	ent	0
Intersection	Period	Control	MOE	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Overall
	AM		LOS	В	В	Α	В	В	В	В	В	Α	В	В	Α	В
26th St. at Pine St.	AIVI	Signal	Delay	16.2	17.0	3.8	11.2	14.1	11.6	16.5	15.7	4.5	18.2	17.0	4.1	14.0
2011 St. at Fille St.	PM	Signal	LOS	В	В	Α	В	С	В	С	В	Α	С	С	Α	В
	FIVI		Delay	14.7	14.3	3.7	18.5	22.3	16.0	21.2	16.9	4.0	25.5	23.6	4.5	16.3
	AM		LOS				С		Α		Α		Α	Α		Α
Pine St. at Caddo St.	Alvi	Cianal	Delay				22.4		3.8		0.8		3.3	7.5		8.2
Pille St. at Caudo St.	PM	Signal	LOS				В		Α		Α		Α	Α		В
	PIVI		Delay				19.3		3.2		6.6		9.1	6.6		10.2
	AM		LOS		Α			Α			Α			Α		Α
Walnut St. at Pine St.	Alvi	Cianal	Delay		5.3			5.3			7.6			5.0		5.6
Valnut St. at Pine St.	PM	Signal	LOS		Α			Α			Α			Α		Α
	PIVI		Delay		5.3			7.1			8.2			7.3		6.5
	AM		LOS		Α						Α			Α		Α
Professional Park Dr.	AIVI	Roundabout	Delay		3.4						6.2			4.0		5.6
at I-30 NB Ramps	PM	Roundabout	LOS		Α						Α			Α		Α
	PIVI		Delay		4.0						6.3			6.6		6.1
	AM		LOS		Α			Α			Α			Α		Α
Red Hill Rd. / Professional Park Dr.	AIVI	Roundabout	Delay		7.0			7.4			7.9			5.9		7.1
at Pine St.	PM	Roundabout	LOS		Α			Α			Α			В		Α
at Pine St.	PIVI		Delay		6.8			9.7			7.5			12.6		8.8
	AM		LOS					Α			Α			Α		Α
Dungage at Ding Ct	AW	Doundahaut	Delay					5.6			2.9			5.5		5.4
Bypass at Pine St.	DM	Roundabout	LOS					Α			Α			Α		Α
	PM		Delay					7.0			6.2			5.7		6.4

<sup>&</sup>lt;sup>1</sup>No volume modeled making this movement

<sup>&</sup>lt;sup>2</sup>Free movement <sup>3</sup>HCM methodology does not calculate LOS due to unconventional configuration

Table 20: Intersection Analysis – 2040 Alternative 2 (Signals) – HCM

Intersection	Time	Control	MOE	EB	Movem	ent	WE	3 Movem		NB	Movem	ent		Movem	ent	- Overall
IIILEISECLIOII	Period	Control		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
			LOS	Α	В	Α	Α	Α	В	С	Α	В	F	Α	В	С
	AM		v/c	0.1	0.5	0.0	0.1	0.0	0.4	0.4	0.0	0.1	1.0	0.0	0.3	
26th St. at Pine St.		Signal	Delay	9.0	14.1	9.8	9.2	0.0	13.4	22.2	0.0	15.7	88.3	0.0	17.3	26.8
2011 01: 411 1110 01:		Olgilai	LOS	В	В	В	Α	Α	В	С	Α	В	F	Α	В	С
	PM		v/c	0.3	0.4	0.1	0.1	0.0	0.7	0.5	0.0	0.1	1.1	0.0	0.3	
			Delay	10.9	13.2	10.3	9.7	0.0	19.9	25.5	0.0	16.0	103.6	0.0	18.0	28.6
			LOS				С		С		Α		Α	Α		Α
	AM		v/c				8.0		0.0		0.2		0.0	0.2		
Pine St. at Caddo St.		Signal	Delay				29.6		21.0		3.6		3.9	3.5		9.3
Fille St. at Caudo St.		Signal	LOS				С		В		Α		Α	Α		В
	PM		v/c				0.9		0.1		0.2		0.0	0.3		
			Delay				26.2		17.4		5.8		6.5	6.1		11.7
			LOS	Α	Α	Α	В	В	В	С	Α	Α	n/a 1	n/a 1	n/a 1	В
	AM		v/c	0.0	0.6	0.6	0.0	0.4	0.4	0.4	0.0	0.0	n/a 1	n/a 1	n/a 1	
Walnut St. at Pine St.		0:	Delay	1.4	3.6	3.8	12.1	15.2	15.1	21.5	0.0	0.0	n/a 1	n/a 1	n/a 1	11.9
wainut St. at Pine St.		Signal	LOS	Α	Α	Α	В	С	Α	В	Α	Α	В	Α	Α	В
	PM		v/c	0.0	0.6	0.6	0.0	0.6	0.0	0.5	0.0	0.0	0.1	0.0	0.0	
PM			Delay	8.5	6.9	7.1	17.4	20.5	0.0	19.9	0.0	0.0	14.2	0.0	0.0	14.6
			LOS	D	Α	Α	Е	Α	Α	С	С	D	С	С	С	В
	AM		v/c	0.9	0.4	0.1	0.5	0.5	0.2	0.2	0.4	0.6	0.1	0.0	0.1	
Professional Park Dr.			Delay	46.3	8.1	6.4	68.1	5.2	4.5	31.7	34.0	41.8	34.9	29.0	30.6	17.1
at Pine St.		Signal	LOS	D	Α	Α	Е	Α	Α	С	С	D	D	С	D	В
	PM		v/c	0.9	0.4	0.1	0.5	0.7	0.3	0.3	0.3	0.4	0.2	0.0	0.6	
			Delay	53.6	8.0	6.4	63.8	5.3	3.5	34.0	33.3	35.8	35.9	29.0	41.9	16.9
			LOS	В		Α					n/a <sup>2</sup>			n/a <sup>2</sup>		Α
	AM		v/c	0.4		0.0					n/a <sup>2</sup>			n/a <sup>2</sup>		
I-30 NB Off-Ramp at		Two-Way	Delay	11.9		8.8					n/a <sup>2</sup>			n/a <sup>2</sup>		7.2
Professional Park Dr.		Stop	LOS	В		Α					n/a <sup>2</sup>			n/a <sup>2</sup>		Α
	PM		v/c	0.3		0.0					n/a <sup>2</sup>			n/a <sup>2</sup>		
			Delay	11.5		8.8					n/a <sup>2</sup>			n/a <sup>2</sup>		6.0
			LOS							Α	Α			Α	Α	Α
	AM		v/c							0.0	0.0			0.0	0.0	
I-30 NB On-Ramp at		Two-Way	Delay							5.0	5.0			5.0	5.0	5.0
Professional Park Dr.		Stop	LOS							A	A			A	A	A
	PM		v/c							0.0	0.0			0.0	0.0	
			Delay							5.0	5.0			5.0	5.0	5.0
<sup>1</sup> No volume modele		41. !	,							3.0	0.0			3.0	0.0	0.0

<sup>&</sup>lt;sup>1</sup>No volume modeled making this movement <sup>2</sup>Free movement

<sup>&</sup>lt;sup>3</sup>HCM methodology does not calculate LOS due to unconventional configuration

Table 21: Intersection Analysis – 2040 Alternative 2 (Signals) – SimTraffic

Time	Control	MOE	EB	Movem		WE	Movem	nent	NB	Movem	ent	SB	Movem		Overall
Period	Control	IVIOL	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Overall
AM		LOS	В	Α	Α	Α	Α	Α	В	В	Α	В	В	Α	Α
7	Signal	Delay	11.8	9.8	1.8	9.7	9.8	5.4	13.3	14.0	5.3	13.1	15.7	3.9	10.0
PM	Olgilai	LOS	В	Α	Α	В	В	В	В	В	Α	С	В	Α	В
1 IVI		Delay	11.4	8.3	3.2	17.4	17.1	12.9	14.3	14.6	3.7	25.2	18.1	3.8	12.7
ΔM		LOS				С		Α		Α		Α	Α		Α
Alvi	Signal	Delay				24.2		2.7		5.1		3.5	3.4		8.6
DM	Olyllai	LOS				С		Α		Α		Α	Α		Α
FIVI		Delay				22.6		4.1		5.5		10.0	4.9		9.8
A N A		LOS	n/a <sup>2</sup>	В	В	Е	В	Α	В	В	Α	n/a 1	n/a 1	n/a 1	В
Alvi	Cianal	Delay	n/a <sup>2</sup>	14.2	13.0	69.7	11.9	5.9	19.2	17.7	3.4	n/a 1	n/a 1	n/a 1	14.1
DM	Signai	LOS	D	В	В	Е	В	n/a 1	В	Α	Α	Α	В	Α	В
PIVI		Delay	39.8	14.5	11.0	60.4	15.8	n/a 1	18.2	0.0	9.5	8.4	19.8	5.8	15.0
4.14		LOS	D	Α	Α	С	С	В	С	С	Α	D	С	Α	В
AM	0'	Delay	35.1	7.4	2.4	28.5	20.6	16.5	29.1	29.9	9.1	42.3	34.3	6.6	17.4
514	Signai	LOS	С	Α	Α	D	С	В	С	С	Α	D	D	Α	В
РМ		Delay	33.6	6.3	2.4	45.7	20.8	14.1	32.2	29.2	5.7	50.1	41.6	10.0	17.5
		LOS	Α		Α					Α			Α		Α
AM	Two-Wav	Delay	6.2		3.1					0.3			0.5		4.0
	Stop	LOS	Α		Α					Α			Α		Α
PM		Delay	6.3		2.0					0.2			0.5		3.4
		LOS							Α	Α			Α	Α	Α
AM at	Two-Way	Delay							8.6	8.6			6.1	2.9	8.3
	Stop	LOS							Α	Α			Α	Α	Α
PM									7.7				6.5		8.1
	Period  AM  PM  AM  PM  AM  PM  AM  PM  AM  PM  P	Period Control  AM Signal PM Two-Way Stop AM Two-Way Stop	Period         Control         MOE           AM         Signal         LOS           PM         Delay           AM         Signal         LOS           PM         Delay           LOS         Delay           LOS<	Period         Control         MOE         Left           AM         Signal         LOS         B           Delay         11.8         LOS         B           Delay         11.4         LOS         Delay           LOS         Delay         LOS         n/a²           Delay         LOS         Delay         n/a²           LOS         Delay         39.8         LOS         Delay           AM         Signal         LOS         Delay         35.1         LOS         C           Delay         33.6         LOS         A         Delay         6.2         LOS         A           PM         Two-Way         Delay         6.3         LOS         A         Delay         6.3           LOS         Two-Way         Delay         Delay         C         Delay         C           Delay         5top         LOS         Delay         Delay         C         Delay         Delay	Period         Control         MOE         Left         Thru           AM         Signal         LOS         B         A           PM         Delay         11.8         9.8         A           LOS         B         A         A         Delay         11.4         8.3           AM         Delay         LOS         Delay         Delay	Period         Control         MOE         Left         Thru         Right           AM         Signal         LOS         B         A         A           PM         Delay         11.8         9.8         1.8           LOS         B         A         A           Delay         11.4         8.3         3.2           LOS         Delay         LOS         Delay           LOS         Delay         Delay         14.2         13.0           LOS         D         B         B         B           Delay         39.8         14.5         11.0         11.0           AM         Signal         LOS         D         A         A         A           Delay         39.8         14.5         11.0         11.0         A         <	Period	Period	Period   Control   MOE   Left   Thru   Right   Left   Thru   Right   Right   Left   Thru   Right   R	Period	Period   Control   MOE   Left   Thru   Right   Right   The left   Thru   Right   Right   Right   Right   The left   Thru   Right   Right   Right   The left   Thru   Right   The left   The left   Thru   Right   The left   The left	Period   Control   MOE   Left   Thru   Right   Left   Thru   Right   Left   Thru   Right   Right   Left   Thru   Right   Rig	Period   Control   MOE   Left   Thru   Right   Thu   Left   Thu   Left	Period   Cohrol   MOE   Left   Thru   Right   Right   Right   Thru   Right   Right	Period   Control   MOE   Left   Thru   Right   Left   Thru   Right   Left   Thru   Right   Left   Thru   Right   Right   Left   Thru   Right   Right

<sup>&</sup>lt;sup>1</sup>No Volume modeled making this movement

<sup>&</sup>lt;sup>2</sup>Low volume modeled making this movement

Table 22: Intersection Analysis – 2040 Alternative 2 (Roundabouts) – HCM

Intersection	Time	Control	MOE	EB	Movem		WE	3 Movem		NB	Movem			Movem	ent	Overall
Intersection	Period	Control		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
			LOS	Α	В	Α	Α	Α	В	С	Α	В	F	Α	В	С
	AM		v/c	0.1	0.5	0.0	0.1	0.0	0.4	0.4	0.0	0.1	1.0	0.0	0.3	
26th St. at Pine St.		Signal	Delay	9.0	14.1	9.8	9.2	0.0	13.4	22.2	0.0	15.7	88.3	0.0	17.3	26.8
	DM		LOS	В	В	В	Α	A	B	C	A	В	F	A	В	С
	PM		v/c	0.3	0.4	0.1	0.1	0.0	0.7	0.5	0.0	0.1	1.1	0.0	0.3	20.0
			Delay LOS	10.9	13.2	10.3	9.7 C	0.0	19.9 C	25.5	0.0 A	16.0	103.6 A	0.0 A	18.0	28.6 A
	AM		v/c				0.8		0.0		0.2		0.0	0.2		A
	AIVI		Delay				29.6		21.0		3.6		3.9	3.5		9.3
Pine St. at Caddo St.		Signal	LOS				C C		B		A.		Α	3.5 A		9.5 B
	PM		v/c				0.9		0.1		0.2		0.0	0.3		D
	1 101		Delay				26.2		17.4		5.8		6.5	6.1		11.7
			LOS	Α		Α	A		A	Α	0.0		A	0.1		A
	AM		v/c	0.4		0.4	0.3		0.3	0.4			0.0			, ,
			Delay	6.0		6.1	6.2		6.0	9.2			5.3			6.5
Walnut St. at Pine St.		Roundabout	LOS	Α		Α	Α		Α	Α			Α			Α
	PM		v/c	0.3		0.4	0.4		0.4	0.4			0.1			
			Delay	6.0		6.0	8.6		8.5	10.0			8.8			7.7
			LOS	Α		Α	В		В	D			Α			В
	AM		v/c	0.4		0.5	0.5		0.5	0.7			0.1			
Professional Park Dr.		Roundabout	Delay	7.2		7.4	11.1		10.7	25.3			6.2			11.2
at Pine St.		Roundabout	LOS	Α		Α	С		С	С			В			В
	PM		v/c	0.4		0.5	0.7		0.7	0.6			0.4			
			Delay	7.3		7.4	17.5		17.5	20.4			14.7			13.6
			LOS		Α						Α			Α		Α
	AM		v/c		0.2						0.1			0.1		
I-30 NB Off-Ramp at		Roundabout	Delay		5.1						4.3			3.3		4.6
Professional Park Dr.			LOS		Α						Α			Α		Α
	PM		v/c		0.2						0.1			0.1		
			Delay		4.7						4.2			3.3		4.3
			LOS							, A					١	Α
	Or. Roundabout	v/c							0					.1		
I-30 NB On-Ramp at Professional Park Dr.		Roundabout	Delay							6					.3	6.6
Professional Park Dr.			LOS							A					4	Α
	PM		v/c							0					.3	7.0
			Delay							6	.9			7	.8	7.2

<sup>&</sup>lt;sup>1</sup>No volume modeled making this movement

<sup>&</sup>lt;sup>2</sup>Free movemen

<sup>&</sup>lt;sup>3</sup>HCM methodology does not calculate LOS due to unconventional configuration

Table 23: Intersection Analysis – 2040 Alternative 2 (Roundabouts) – SIDRA

Intersection	Time	Control	MOE	EB	Movem	nent	WE	Movem	nent	NE	Movem	ent	SB	Movem	ent	Overall
intersection	Period	Control	MOE	Left	Thru	Right	Overall									
	AM		LOS	Α	Α	Α	В	В	Α	В	В	Α	В	В	Α	В
26th St. at Pine St.	VIAI	Signal	Delay	9.8	7.7	2.5	10.2	12.2	4.6	14.5	14.5	4.1	18.7	16.1	6.9	10.4
20(1) 51. 8(1) 1116 51.	PM	Signal	LOS	В	В	Α	В	В	В	В	В	Α	В	В	Α	В
	I IVI		Delay	14.5	11.0	2.8	19.6	17.2	13.7	20.0	14.5	4.0	17.4	15.9	4.3	13.6
	AM		LOS				С		Α		Α		С	Α		Α
Caddo St. at Pine St.	Alvi	Signal	Delay				24.0		2.2		3.1		20.1	6.1		8.9
Caudo St. at Pille St.	PM	Signai	LOS				В		Α		Α		В	Α		В
	PIVI		Delay				20.0		4.0		5.9		15.4	9.0		11.2
	AM		LOS		Α			Α			Α			Α		Α
Walnut Ct at Dina Ct	Alvi	Roundabout	Delay		5.3			5.3			7.6			5.0		5.6
Walnut St. at Pine St.	DM	Roundabout	LOS		Α			Α			Α			Α		Α
	FIVI		Delay		5.3			7.1			8.2			7.3		6.5
	AM		LOS		Α			Α			В			Α		Α
Professional Park Dr.	Alvi	Roundabout	Delay		6.3			8.6			15.7			5.5		8.4
at Pine St.	PM	Roundabout	LOS		Α			В			В			В		Α
	PIVI		Delay		6.3			12.0			13.7			11.0		9.9
	AM		LOS		Α						Α			Α		Α
I-30 NB Off-Ramp at	AIVI	Roundabout	Delay		4.7						4.1			3.2		4.3
Professional Park Dr.	PM	Roundabout	LOS		Α						Α			Α		Α
	PIVI		Delay		4.4						4.0			3.2		4.0
AM	A.N.4		LOS								Α			Α		Α
I-30 NB On-Ramp at	AIVI	Roundabout	Delay								6.1			4.0		5.9
Professional Park Dr.	PM	Rounuabout	LOS								Α			Α		Α
	PIVI		Delay								6.2			6.7		6.4

<sup>&</sup>lt;sup>1</sup>No volume modeled making this movement

<sup>&</sup>lt;sup>2</sup>Free movement <sup>3</sup>HCM methodology does not calculate LOS due to unconventional configuration

Table 24: Intersection Analysis – 2040 Alternative 3 (Signals) – HCM

1.1	Time	011	МОЕ	EB	Movem	ent	WE	3 Movem	ent	NE	Movem	ent	SB	Movem	ent	<b>^</b>
Intersection	Period	Control	MOE	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Overall
			LOS	Α	В	Α	Α	Α	В	С	Α	В	F	Α	В	С
	AM		v/c	0.1	0.5	0.0	0.1	0.0	0.4	0.4	0.0	0.1	1.0	0.0	0.3	
26th St. at Pine St.		Cianal	Delay	9.0	14.1	9.8	9.2	0.0	13.4	22.2	0.0	15.7	88.3	0.0	17.3	26.8
Zoth St. at Pine St.		Signal	LOS	В	В	В	Α	Α	В	С	Α	В	F	Α	В	С
	PM		v/c	0.3	0.4	0.1	0.1	0.0	0.7	0.5	0.0	0.1	1.1	0.0	0.3	
			Delay	10.9	13.2	10.3	9.7	0.0	19.9	25.5	0.0	16.0	103.6	0.0	18.0	28.6
			LOS				С		С		Α		Α	Α		Α
	AM		v/c				0.8		0.0		0.2		0.0	0.2		
Pine St. at Caddo St.		Signal	Delay				29.6		21.0		3.6		3.9	3.5		9.3
Fille St. at Caudo St.		Sigilal	LOS				С		В		Α		Α	Α		В
	PM		v/c				0.9		0.1		0.2		0.0	0.3		
			Delay				26.2		17.4		5.8		6.5	6.1		11.7
			LOS	Α	Α	Α	В	В	В	С	Α	Α	n/a 1	n/a <sup>1</sup>	n/a 1	Α
A	AM		v/c	0.0	0.6	0.6	0.0	0.4	0.4	0.4	0.0	0.0	n/a 1	n/a 1	n/a 1	
Walnut St. at Pine St.		Signal	Delay	1.4	3.5	3.7	12.1	15.2	15.1	21.6	0.0	0.0	n/a 1	n/a 1	n/a 1	10.0
Wallut St. at 1 life St.		Signal	LOS	Α	Α	Α	В	С	n/a 1	В	Α	Α	В	Α	Α	В
	PM		v/c	0.0	0.6	0.6	0.0	0.6	n/a 1	0.5	0.0	0.0	0.1	0.0	0.0	
			Delay	8.5	6.7	7.0	17.4	20.5	n/a 1	19.9	0.0	0.0	14.2	0.0	0.0	14.5
			LOS	D	Α	Α	Е	Α	Α	С	С	С	С	С	С	Α
	AM		v/c	0.9	0.5	0.1	0.5	0.5	0.1	0.3	0.0	0.0	0.1	0.0	0.2	
Red Hill Rd. / Professional Park Dr.		Signal	Delay	40.6	0.6	0.2	61.9	1.4	0.7	32.4	28.6	28.6	29.3	28.6	30.6	7.3
at Pine St.		Olgilai	LOS	В	В	В	Α	Α	Α	С	С	С	С	С	С	В
	PM		v/c	0.2	0.4	0.1	0.0	0.6	0.1	0.3	0.0	0.0	0.1	0.0	0.6	
			Delay	13.4	14.7	10.6	2.6	8.0	0.1	29.2	24.7	24.7	26.0	24.7	34.9	10.6
			LOS	D	Α			Α	Α	С	Α	D				В
	AM		v/c	0.8	0.5			0.5	0.2	0.2	0.0	0.6				
I-30 NB Ramps at		Signal	Delay	48.4	8.4			0.8	0.6	28.1	0.0	38.6				11.1
Pine St.		Oigilal	LOS	D	Α			С	С	С	Α	D				С
	PM		v/c	0.9	0.4			0.8	0.4	0.3	0.0	0.5				
	PM		Delay	55.0	6.0			34.9	26.0	33.5	0.0	37.5				26.4

<sup>&</sup>lt;sup>1</sup>No volume modeled making this movement

<sup>&</sup>lt;sup>2</sup>Free movement <sup>3</sup>HCM methodology does not calculate LOS due to unconventional configuration

Table 25: Intersection Analysis – 2040 Alternative 3 (Signals) – SimTraffic

1.1	Time	011	МОЕ	EB	Movem	ent	WE	Movem	nent	NE	Movem	ent	SE	Movem	ent	<b>^</b>
Intersection	Period	Control	MOE	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Overall
	AM		LOS	Α	Α	Α	В	В	Α	В	В	Α	В	В	Α	Α
26th St. at Pine St.	AIVI	0:	Delay	8.6	7.9	2.4	11.3	10.5	8.6	14.6	12.6	4.2	15.4	16.2	3.7	9.7
Zoth St. at Pine St.	PM	Signal	LOS	В	Α	Α	В	В	В	С	В	Α	В	В	Α	В
	PIVI		Delay	10.3	7.6	1.6	16.5	16.3	11.4	20.5	18.3	4.6	16.5	17.1	4.9	11.9
	AM		LOS				С		Α		Α		n/a <sup>2</sup>	Α		Α
Caddo St. at Pine St.	AIVI	Cianal	Delay				22.6		4.8		3.9		n/a <sup>2</sup>	3.5		7.8
Caddo St. at Pine St.	PM	Signal	LOS				В		Α		Α		В	Α		Α
	PIVI		Delay				17.0		2.9		5.3		19.4	5.9		8.6
	AM		LOS	n/a 1	В	Α	С	В	Α	С	В	Α	n/a 1	n/a 1	n/a 1	В
Walnut St. at Pine St.	AIVI	Signal	Delay	n/a 1	10.9	9.1	29.6	11.7	1.8	22.7	15.1	6.1	n/a 1	n/a 1	n/a 1	12.8
Walliut St. at Fille St.	PM	Signal	LOS	В	С	В	D	В	n/a 1	В	n/a <sup>2</sup>	В	В	Α	Α	В
	FIVI		Delay	16.0	20.1	15.6	39.4	17.0	n/a 1	19.0	n/a <sup>2</sup>	15.4	12.2	1.7	7.0	17.8
	AM		LOS	D	Α	Α	D	В	Α	С	С	Α	С	С	Α	В
Red Hill Rd. / Professional Park Dr.	AIVI	Signal	Delay	35.9	9.8	4.6	50.1	12.9	4.3	31.0	30.9	4.0	34.0	26.4	6.4	13.9
at Pine St.	PM	Signal	LOS	D	Α	Α	С	Α	Α	С	С	Α	С	Α	В	В
	FIVI		Delay	36.5	9.3	3.8	24.7	7.7	4.4	25.3	32.9	2.3	22.3	9.1	13.3	10.3
	AM		LOS	D	Α			С	Α	С	Е	В				В
I-30 NB Ramps at	Alvi	Signal	Delay	42.0	6.8			23.3	9.3	29.3	64.7	11.6				15.4
Pine St.	PM	Signal	LOS	D	Α			В	Α	С	n/a 1	Α				В
	FIVI		Delay	38.1	5.7			18.2	9.0	30.6	n/a 1	7.8				15.0

<sup>&</sup>lt;sup>1</sup>No Volume modeled making this movement

<sup>&</sup>lt;sup>2</sup>Low volume modeled making this movement

Table 26: Intersection Analysis – 2040 Alternative 3 (Roundabouts) – HCM

latana aftan	Time	Oznataval	МОЕ	EE	3 Movem	ent	WE	Movem	ent	NE	Movem	ent	SE	Movem	ent	0
Intersection	Period	Control	MOE	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Overall
			LOS	Α	В	Α	Α	Α	В	С	Α	В	F	Α	В	С
	AM		v/c	0.1	0.5	0.0	0.1	0.0	0.4	0.4	0.0	0.1	1.0	0.0	0.3	
26th St. at Pine St.		Cianal	Delay	9.0	14.1	9.8	9.2	0.0	13.4	22.2	0.0	15.7	88.3	0.0	17.3	26.8
Zoth St. at Pine St.		Signal	LOS	В	В	В	Α	Α	В	С	Α	В	F	Α	В	С
	PM		v/c	0.3	0.4	0.1	0.1	0.0	0.7	0.5	0.0	0.1	1.1	0.0	0.3	
			Delay	10.9	13.2	10.3	9.7	0.0	19.9	25.5	0.0	16.0	103.6	0.0	18.0	28.6
			LOS				С		С		Α		Α	Α		Α
	AM		v/c				0.8		0.0		0.2		0.0	0.2		
Pine St. at Caddo St.		Signal	Delay				29.6		21.0		3.6		3.9	3.5		9.3
Pille St. at Caudo St.		Signai	LOS				С		В		Α		Α	Α		В
	PM		v/c				0.9		0.1		0.2		0.0	0.3		
			Delay				26.2		17.4		5.8		6.5	6.1		11.7
			LOS		Α			Α			Α			Α		Α
	AM		v/c		0.7			0.3			0.3			0.0		
Walnut St. at Pine St.		Roundabout	Delay		6.0			6.1			9.2			0.0		6.5
Walliut St. at Fille St.		Roundabout	LOS		Α			Α			Α			Α		Α
	PM		v/c		0.3			0.4			0.3			0.3		
			Delay		6.0			8.5			10.0			8.8		7.7
			LOS		Α			Α			В			Α		Α
Red Hill Rd. /	AM		v/c		0.3			0.4			0.3			0.3		
Professional Park Dr.		Roundabout	Delay		7.8			8.2			10.4			6.9		8.0
at Pine St.			LOS		Α			Α			Α			С		Α
u	PM		v/c		0.4			0.5			0.2			0.6		
			Delay		6.9			9.5			8.9			20.9		9.6
AM			LOS		Α			Α			С					Α
	AM		v/c		0.4			0.5			0.2					
I-30 NB Ramps at		Roundabout	Delay		7.0			7.5			20.0					8.7
Pine St.	D14		LOS		A			С			С					В
	PM		v/c		0.4			0.7			0.4					40.0
		//	Delay		6.8			16.2			15.1					12.3

<sup>&</sup>lt;sup>1</sup>No volume modeled making this movement <sup>2</sup>Free movement

 $<sup>^3\</sup>mbox{HCM}$  methodology does not calculate LOS due to unconventional configuration

Table 27: Intersection Analysis – 2040 Alternative 3 (Roundabouts) – SIDRA

Intersection	Time	Control	MOE	EB	3 Movem	ent	WE	Movem	ent	NE	Movem	ent	SE	Movem	ent	Overall
intersection	Period	Control	MOE	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Overall
	AM		LOS	В	Α	Α	В	Α	Α	В	В	Α	С	В	Α	Α
26th St. at Pine St.	Alvi	Signal	Delay	10.4	6.8	1.3	14.2	8.4	6.4	18.5	13.4	3.8	20.4	16.5	3.9	9.4
2001 00: 001 1110 00:	РМ	Oigilai	LOS	В	Α	Α	В	В	В	В	В	Α	В	В	Α	В
	F IVI		Delay	12.1	6.2	2.0	19.3	17.5	11.9	18.3	15.4	3.6	17.0	15.6	4.8	12.2
	AM		LOS				С		Α		Α		В	Α		Α
Caddo St. at Pine St.	Alvi	Signal	Delay				23.4		3.4		3.0		16.0	3.5		7.6
Odddo ot. at i ille ot.	РМ	Olgilai	LOS				С		Α		Α		В	Α		Α
			Delay				21.8		3.0		4.5		18.2	4.2		8.7
	AM		LOS		Α			Α			Α			Α		Α
Walnut Ct at Dina Ct	Alvi	Davidahavit	Delay		5.3			5.4			7.7			5.0		5.7
Walnut St. at Pine St.	РМ	Roundabout	LOS		Α			Α			Α			Α		Α
	FIVI		Delay		5.3			7.1			8.1			7.3		6.5
	AM		LOS		Α			Α			Α			Α		Α
Red Hill Rd. /	AW	Davidahasi	Delay		6.6			6.9			8.4			6.0		6.8
Professional Park Dr. at Pine St.	DM	Roundabout	LOS		Α			Α			Α			В		Α
at Pille St.	PM		Delay		5.9			7.8			7.4			14.2		7.7
	AM		LOS		Α			Α			В					Α
I-30 NB Ramps at	AIVI	Roundabout	Delay		6.0			6.4			13.6					7.1
Pine St.	PM	Roundabout	LOS		Α			В			В					Α
	FIVI		Delay		5.9			11.3			11.2					9.1

<sup>&</sup>lt;sup>1</sup>No volume modeled making this movement

<sup>&</sup>lt;sup>2</sup>Free movement

<sup>&</sup>lt;sup>3</sup>HCM methodology does not calculate LOS due to unconventional configuration

Table 28: Intersection Analysis – 2040 Alternative 4 (Signals) – HCM

Intersection	Time	Control	MOE	EB	Movem	ent	WE	3 Movem	ent	NB	Movem	ent	SB	Movem	ent	Overall
Intersection	Period	Control	IVIOE	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Overall
			LOS	Α	В	Α	Α	Α	В	С	Α	В	F	Α	В	С
	AM		v/c	0.1	0.5	0.0	0.1	0.0	0.4	0.4	0.0	0.1	1.0	0.0	0.3	
26th St. at Pine St.		Signal	Delay	9.0	14.1	9.8	9.2	0.0	13.4	22.2	0.0	15.7	88.3	0.0	17.3	26.8
2001 00. 001 1110 00.		Oigilai	LOS	В	В	В	Α	Α	В	С	Α	В	F	Α	В	С
	PM		v/c	0.3	0.4	0.1	0.1	0.0	0.7	0.5	0.0	0.1	1.1	0.0	0.3	
			Delay	10.9	13.2	10.3	9.7	0.0	19.9	25.5	0.0	16.0	103.6	0.0	18.0	28.6
			LOS				С		С		Α		Α	Α		Α
	AM		v/c				8.0		0.0		0.2		0.0	0.2		
Pine St. at Caddo St.		Signal	Delay				29.6		21.0		3.6		3.9	3.5		9.3
Time of at oddie of.		Olgilai	LOS				С		В		Α		Α	Α		В
	PM		v/c				0.9		0.1		0.2		0.0	0.3		
			Delay				26.2		17.4		5.8		6.5	6.1		11.7
			LOS	В	В	В	В	В	В	С	С	С	В	В	В	В
	AM		v/c	0.0	0.0	0.0	0.0	0.0	0.5	0.0	0.1	0.0	0.6	0.1	0.0	
Bypass at Pine St.		One-Way	Delay	19.1	18.7	18.8	19.1	18.7	10.1	27.2	28.6	27.8	15.7	12.2	11.9	13.8
Dypass at 1 inc ot.		Stop	LOS	С	С	С	С	С	В	С	С	С	С	В	В	С
	PM		v/c	0.0	0.0	0.0	0.0	0.0	0.7	0.0	0.1	0.0	0.9	0.1	0.0	
			Delay	21.7	21.2	21.2	21.5	21.2	12.3	28.0	29.7	28.6	32.4	10.4	10.1	21.0
			LOS	С	С	С	С	С	С	В	В	В	В	В	В	С
100 ND D	AM		v/c	0.6	8.0	0.3	0.6	0.7	0.1	0.1	0.1	0.3	0.0	0.0	0.1	
I-30 NB Ramps / Professional Park Dr.		Signal	Delay	21.7	33.4	23.5	23.8	28.5	21.3	17.3	18.3	15.9	18.0	19.2	14.4	27.2
at Pine St.		Signal	LOS	С	С	С	С	С	В	В	В	В	В	С	С	С
	PM		v/c	0.4	0.9	0.5	0.7	0.9	0.1	0.2	0.0	0.2	0.1	0.1	0.3	
			Delay	23.3	34.7	26.3	25.4	29.4	16.3	18.6	19.6	15.3	18.9	21.3	22.0	28.3
			LOS	В	С	С	С	В	В	В	Α	Α	n/a 1	n/a 1	n/a 1	В
	AM		v/c	0.0	8.0	8.0	0.0	0.5	0.5	0.3	0.0	0.0	n/a 1	n/a 1	n/a 1	
Walnut St. at Pine St.		Signal	Delay	17.2	21.5	21.7	24.6	14.7	14.7	11.5	0.0	0.0	n/a 1	n/a 1	n/a 1	17.9
vvaiilut St. at Fille St.		Signal	LOS	С	С	С	С	В	n/a 1	В	Α	Α	Α	Α	Α	В
	PM		v/c	0.0	0.8	8.0	0.0	0.8	n/a 1	0.4	0.0	0.0	0.1	0.0	0.0	
			Delay	24.0	22.0	22.0	25.1	19.6	n/a 1	11.4	0.0	0.0	7.8	0.0	0.0	19.8

<sup>&</sup>lt;sup>1</sup>No volume modeled making this movement <sup>2</sup>Free movement

<sup>&</sup>lt;sup>3</sup>HCM methodology does not calculate LOS due to unconventional configuration

Table 29: Intersection Analysis – 2040 Alternative 4 (Signals) – SimTraffic

Intersection	Time	Control	MOE	EB	Movem	ent	WE	3 Movem	ent	NB	Movem	ent	SE	Movem	ent	Overall
IIILEISECIIOII	Period	Control	WOE	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Overall
	AM		LOS	В	В	Α	В	В	Α	В	В	Α	В	В	Α	В
26th St. at Pine St.	Alvi	Cianal	Delay	15.1	17.9	2.8	12.4	14.1	6.7	15.8	16.9	5.0	18.7	17.5	4.0	14.6
Zotii St. at Pille St.	PM	Signal	LOS	В	В	Α	В	В	В	В	В	Α	В	В	Α	В
	FIVI		Delay	14.7	14.7	3.0	16.4	19.4	14.9	18.2	15.5	3.6	18.9	18.1	5.1	14.9
	AM		LOS				С		Α		Α		n/a 1	Α		Α
0-44-04-40:04	AIVI	0:	Delay				17.7		2.9		0.9		n/a 1	7.9		7.1
Caddo St. at Pine St.	DM	Signal	LOS				С		Α		Α		В	В		Α
	PM		Delay				16.7		2.4		2.1		10.5	11.1		9.7
	414		LOS	С	С	Α	С	С	Α	Α	С	Α	В	Α	Α	В
D 1D: 01	AM	One-Way	Delay	25.9	22.1	3.1	30.3	29.6	7.6	7.4	20.4	5.1	18.0	3.1	2.9	13.2
Bypass at Pine St.	DM	Stop	LOS	С	С	Α	D	В	Α	Α	С	Α	D	Α	Α	С
	PM		Delay	32.9	27.9	3.1	45.8	11.5	9.4	9.0	23.4	2.0	37.6	3.6	3.5	20.6
	414		LOS	F	F	В	С	В	Α	С	С	В	С	D	Α	D
I-30 NB Ramps /	AM	0: 1	Delay	90.2	95.2	10.4	24.3	17.4	4.4	31.3	25.3	14.6	32.0	38.8	6.6	49.8
Professional Park Dr. at Pine St.	DM	Signal	LOS	Е	Е	В	С	В	Α	С	D	Α	С	В	Α	С
att mo ot.	PM		Delay	65.5	72.9	11.4	27.2	16.6	3.9	24.0	35.7	10.0	23.8	19.8	7.5	33.8
	414		LOS	Е	В	В	Е	В	n/a <sup>2</sup>	В	Α	Α	n/a 1	n/a 1	n/a 1	В
Welend Ot at Dia Ot	AM	0:	Delay	59.6	17.5	15.8	58.5	18.2	n/a²	11.3	9.7	7.5	n/a 1	n/a 1	n/a 1	16.7
Walnut St. at Pine St.	DM	Signal	LOS	Е	В	С	Е	С	n/a 1	В	D	Α	Α	В	Α	С
	PM		Delay	57.5	19.2	21.0	57.5	24.7	n/a <sup>1</sup>	11.9	36.2	7.1	8.5	12.0	9.1	20.3

<sup>&</sup>lt;sup>1</sup>No Volume modeled making this movement

<sup>&</sup>lt;sup>2</sup>Low volume modeled making this movement

Table 30: Intersection Analysis – 2040 Alternative 4 (Roundabouts) – HCM

late are effect	Time	Combani	МОЕ	EB	Movem	ent	WE	Moven	nent	NB	Movem	ent	SB	Movem	ent	0
Intersection	Period	Control	MOE	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Overall
			LOS	Α	В	Α	Α	Α	В	С	Α	В	F	Α	В	С
	AM		v/c	0.1	0.5	0.0	0.1	0.0	0.4	0.4	0.0	0.1	1.0	0.0	0.3	
26th St. at Pine St.		Signal	Delay	9.0	14.1	9.8	9.2	0.0	13.4	22.2	0.0	15.7	88.3	0.0	17.3	26.8
Zour St. at Fille St.		Sigilal	LOS	В	В	В	В	Α	С	С	Α	В	F	Α	В	С
	PM		v/c	0.3	0.4	0.1	0.1	0.0	0.7	0.5	0.0	0.1	1.1	0.0	0.3	
			Delay	12.3	13.3	10.3	10.1	0.0	21.5	25.5	0.0	16.0	103.6	0.0	18.0	29.2
			LOS				С		С		Α		Α	Α		Α
	AM		v/c				0.8		0.0		0.2		0.0	0.2		
D: Ot -t O11- Ot		0:	Delay				29.6		21.0		3.6		3.9	3.5		9.3
Pine St. at Caddo St.		Signal	LOS				С		В		Α		Α	Α		В
	PM		v/c				0.9		0.1		0.2		0.0	0.3		
			Delay				26.2		17.1		5.8		6.5	6.1		11.5
			LOS		Α			Α			Α			Α		Α
	AM		v/c		0.7			0.3			0.3			0.0		
Dumana at Dina Ct		Daumdahaut	Delay		7.0			6.1			7.5			6.7		6.5
Bypass at Pine St.		Roundabout	LOS		Α			Α			Α			Α		Α
	PM		v/c		0.0			0.5			0.1			0.4		
			Delay		6.6			8.2			7.3			6.5		7.4
			LOS		Α			Α			С			Α		Α
	AM		v/c		0.5			0.5			0.5			0.1		
I-30 NB Ramps / Professional Park Dr.		Roundabout	Delay		9.2			8.5			16.1			7.2		9.7
at Pine St.		Roundabout	LOS		В			В			В			С		В
att mo ou	PM		v/c		0.6			0.6			0.4			0.6		
			Delay		10.7			10.1			11.8			23.9		11.7
			LOS	В	С	С	С	В	В	В	Α	Α	n/a 1	n/a <sup>1</sup>	n/a 1	В
Walant Ot at Diag Ot	AM		v/c	0.0	8.0	8.0	0.0	0.5	0.5	0.3	0.0	0.0	n/a 1	n/a 1	n/a 1	
		Cianal	Delay	17.2	22.5	22.8	24.6	14.7	14.7	11.5	0.0	0.0	n/a 1	n/a <sup>1</sup>	n/a 1	18.5
Walnut St. at Pine St.		Signal	LOS	С	С	С	С	В	n/a 1	В	Α	Α	Α	Α	Α	С
	PM		v/c	0.1	0.8	8.0	0.0	0.8	n/a 1	0.4	0.2	0.5	0.1	0.0	0.0	
			Delay	24.1	24.7	25.1	25.1	19.6	n/a 1	11.4	0.0	0.0	7.8	0.0	0.0	20.4

<sup>&</sup>lt;sup>1</sup>No volume modeled making this movement <sup>2</sup>Free movement

<sup>&</sup>lt;sup>3</sup>HCM methodology does not calculate LOS due to unconventional configuration

Table 31: Intersection Analysis – 2040 Alternative 4 (Roundabouts) – SIDRA

Intersection	Time	Control	MOE	EB	Movem	ent	WE	3 Movem	nent	NB	Movem	ent	SB	Movem	ent	Overall
mersection	Period	Control	IVIUE	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Overall
	AM		LOS	В	В	Α	В	В	Α	В	В	Α	В	В	Α	В
26th St. at Pine St.	AIVI	Signal	Delay	18.1	18.5	2.9	16.2	16.7	8.0	17.1	11.9	4.6	16.3	17.4	4.2	15.0
Zour St. at Pille St.	PM	Sigilal	LOS	В	В	Α	В	С	В	В	В	Α	В	В	Α	В
	PIVI		Delay	14.9	16.7	5.0	17.8	23.4	19.1	18.0	18.8	3.6	19.5	17.6	4.0	17.0
	A N 4		LOS				С		Α		Α		Α	Α		Α
O-44- Ot -t D: Ot	AM	0:	Delay				23.5		3.1		0.8		7.6	6.3		7.9
Caddo St. at Pine St.	DM	Signal	LOS				В		Α		Α		В	В		В
	PM		Delay				17.0		2.5		1.9		15.2	13.3		10.6
	A N 4		LOS	С	В	В	Е	В	В	В	В	Α	n/a 1	n/a 1	n/a 1	В
Walnut St. at Pine St.	AM	0:	Delay	21.3	14.4	11.4	59.3	15.5	18.6	10.1	18.7	6.7	n/a 1	n/a 1	n/a 1	13.9
walnut St. at Pine St.	PM	Signal	LOS	Е	В	В	D	В	n/a 1	В	n/a <sup>2</sup>	Α	Α	Α	Α	В
	PIVI		Delay	55.1	15.6	14.6	50.4	19.1	n/a 1	12.1	n/a <sup>2</sup>	7.3	6.6	4.8	3.7	16.2
	A N 4		LOS		Α			Α			Α			Α		Α
D 15: 01	AM	D 11 (	Delay		5.3			5.3			7.6			5.0		5.6
Bypass at Pine St.	DM	Roundabout	LOS		Α			Α			Α			Α		Α
	PM		Delay		6.0			7.1			5.6			7.3		6.5
I-30 NB Ramps /			LOS		Α			Α			В			Α		Α
	AM	D 11 (	Delay		7.6			7.0			11.7			6.2		7.8
Professional Park Dr. at Pine St.	DM	Roundabout	LOS		Α			Α			Α			В		Α
at i ille ot.	PM		Delay		8.5			8.1			9.3			15.5		9.0

<sup>&</sup>lt;sup>1</sup>No volume modeled making this movement <sup>2</sup>Free movement

<sup>&</sup>lt;sup>3</sup>HCM methodology does not calculate LOS due to unconventional configuration

Table 32: Intersection Analysis – 2040 New Interchange – HCM

Intersection	Time	Control	MOE	EB	Movem	nent	WB	Moven	nent	NB	Movem	nent	SB	Moven	nent	Overall
IIILETSECTION	Period	Control	IVIOE	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Overall
	AM		v/c	0.1	0.5	0.0	0.1	0.0	0.4	0.4	0.0	0.1	1.0	0.0	0.3	
	Alvi		Delay	9.0	14.2	9.8	9.2	0.0	13.4	22.2	0.0	15.7	88.3	0.0	17.3	26.8
26th St. at Pine St.		Signal	LOS	В	В	В	Α	Α	В	С	Α	В	F	Α	В	С
	PM		v/c	0.3	0.4	0.1	0.1	0.0	0.7	0.5	0.0	0.1	1.1	0.0	0.3	
			Delay	10.9	13.2	10.2	9.6	0.0	19.8	25.6	0.0	16.0	103.7	0.0	18.1	28.6
			LOS				С		С		Α		Α	Α		Α
	AM		v/c				0.8		0.0		0.2		0.0	0.2		
Pine St. at Caddo		Signal	Delay				29.6		21.0		3.6		3.9	3.5		9.3
St.		Oigilai	LOS				С		В		Α		Α	Α		В
	PM		v/c				0.9		0.1		0.2		0.0	0.3		
			Delay				28.2		17.2		5.7		6.4	6.1		12.0
			LOS	Α	Α	Α	Α	Α	Α	В	Α	Α	n/a 1	n/a 1	n/a 1	Α
Walnut St. at Pine	AM		v/c	0.0	0.5	0.5	0.0	0.3	0.3	0.6	0.0	0.0	n/a 1	n/a <sup>1</sup>	n/a 1	
		Signal	Delay	6.3	8.1	8.2	9.1	6.0	6.0	19.2	0.0	0.0	n/a 1	n/a <sup>1</sup>	n/a 1	9.1
St. / Driveway		Oigila	LOS	В	Α	Α	В	Α	n/a 1	В	Α	Α	В	Α	Α	В
	PM		v/c	0.0	0.5	0.5	0.0	0.5	0.0	0.6	0.0	0.0	0.1	0.0	0.0	
			Delay	10.1	9.5	9.7	10.5	8.2	n/a 1	18.1	0.0	0.0	13.5	0.0	0.0	10.4
			LOS	Α	Α	Α	Α	Α	Α	С	С	С	С	С	С	Α
Red Hill Rd. /	AM		v/c	0.4	0.4	0.0	0.0	0.4	0.1	0.0	0.1	0.1	0.1	0.1	0.5	
Professional Park		Signal	Delay	3.7	2.4	1.4	4.3	6.0	4.5	26.6	26.4	26.5	26.9	26.4	31.2	5.1
Dr. at Pine St.		Oigila	LOS	Α	Α	Α	Α	В	Α	С	С	С	С	С	С	В
	PM		v/c	0.2	0.4	0.0	0.0	0.7	0.1	0.0	0.0	0.0	0.1	0.0	0.8	
			Delay	7.7	5.1	3.4	6.7	11.8	7.0	20.3	20.1	20.1	21.0	20.1	29.4	10.9
			LOS	Α	Α			Α	Α	С	Α	С				Α
	AM		v/c	0.3	0.5			0.4	0.2	0.3	0.0	0.7				
I-30 NB Ramps at		Signal	Delay	5.2	4.6			8.7	7.3	21.2	0.0	26.9				8.2
Pine St.		Oignai	LOS	Α	Α			Α	Α	D	Α	С				Α
	PM		v/c	0.6	0.3			0.6	0.2	0.7	0.0	0.6				
			Delay	7.7	2.6			7.6	5.5	35.8	0.0	34.8				7.6

<sup>&</sup>lt;sup>1</sup>No volume modeled making this movement

<sup>&</sup>lt;sup>2</sup>Free movement <sup>3</sup>HCM methodology does not calculate LOS due to unconventional configuration

Table 33: Intersection Analysis – 2040 New Interchange – SimTraffic

lute ve e etie v	Time	Cambral	MOE	EB	Moven	nent	WB	Moven	nent	NB	Moven	nent	SB	Moven	nent	Ouerell
Intersection	Period	Control	IVIOE	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Overall
	AM		LOS	В	Α	Α	В	В	Α	В	В	Α	В	В	Α	Α
26th St. at Pine St.	Alvi	Signal	Delay	10.1	7.0	2.8	12.5	10.4	9.7	17.4	17.3	4.1	19.2	14.0	3.4	9.7
ZOUT St. at FINE St.	PM	Signal	LOS	В	Α	Α	В	В	В	В	В	Α	В	В	Α	В
	L IAI		Delay	12.0	5.9	2.5	16.1	14.3	10.3	15.3	12.7	3.8	18.1	15.8	4.8	11.2
	AM		LOS				С		Α		Α		n/a²	Α		Α
Caddo St. at Pine	Alvi	Signal	Delay				21.5		2.8		5.1		n/a²	3.2		8.1
St.	PM	Signal	LOS				В		Α		Α		Α	Α		Α
	T IVI		Delay				16.5		3.2		6.1		5.7	4.7		8.6
	AM		LOS	n/a 1	Α	Α	В	Α	n/a <sup>2</sup>	В	n/a <sup>2</sup>	Α	n/a 1	n/a 1	n/a 1	Α
Walnut St. at Pine	Alvi	Signal	Delay	n/a 1	5.5	4.2	19.7	4.4	n/a <sup>2</sup>	17.5	n/a <sup>2</sup>	10.0	n/a <sup>1</sup>	n/a 1	n/a 1	7.0
St. / Driveway	PM	Olgilai	LOS	В	Α	Α	n/a <sup>2</sup>	Α	n/a 1	В	n/a <sup>2</sup>	Α	В	В	Α	Α
	L IAI		Delay	12.6	6.2	5.6	n/a <sup>2</sup>	5.2	n/a 1	16.2	n/a <sup>2</sup>	9.4	14.4	16.0	5.2	7.4
D 1155 D1 /	AM		LOS	Α	Α	Α	Α	Α	Α	В	С	Α	С	n/a <sup>2</sup>	Α	Α
Red Hill Rd. / Professional Park	Alvi	Signal	Delay	8.4	1.6	2.2	8.5	2.7	2.2	19.6	24.7	2.1	27.6	n/a <sup>2</sup>	4.0	3.3
Dr. at Pine St.	PM	Signal	LOS	В	Α	n/a²	В	Α	Α	В	С	Α	С	С	Α	В
	L IAI		Delay	10.2	3.2	2.4	10.7	5.0	2.4	19.1	23.6	0.7	25.3	22.2	8.5	12.0
	AM		LOS	Α	Α			Α	Α	В	С	Α				Α
I-30 NB Ramps at	Alvi	Signal	Delay	6.7	2.0			3.5	1.8	19.9	26.9	7.0				3.8
Pine St.	PM	Signal	LOS	В	Α			Α	Α	С	n/a 1	Α				Α
	F IVI		Delay	11.5	3.1			7.1	4.8	32.2	n/a 1	4.7				7.2

<sup>&</sup>lt;sup>1</sup>No Volume modeled making this movement

# **2040 Intersection Analysis, Eastern Alternative Conditions**

The results from the eastern intersection analyses are shown in **Table 34** through **Table 37**. The complete results are provided in **Appendix B - Intersection Operational Analysis Results.** 

The results of the operational analysis of the Eastern Action Alternatives indicate similar delays for each scenario. Generally, the overall intersection LOS for the intersections on the eastern end of study area operate acceptably during AM and PM peak hours.

<sup>&</sup>lt;sup>2</sup>Low volume modeled making this movement

Table 34: Intersection Analysis – 2040 Alternative E and F – HCM

late was fine	Torr Deviced	Overteel	MOE	EB	Moven	nent	WB	Mover	nent	NB	Moven	nent	SB	Movem	ent	0
Intersection	Time Period	Control	MOE	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Overall
			LOS		n/a <sup>1</sup>					Α	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	Α
	AM		v/c		n/a <sup>1</sup>					0.0	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	
Hwy 67 at		One-Way Stop	Delay		n/a <sup>1</sup>					7.4	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	0.0
Siplast Rd.		One-way Stop	LOS		В					Α	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	Α
	PM		v/c		0.0					0.0	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	
			Delay		11.4					7.8	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	0.2
			LOS		С					Α	n/a <sup>2</sup>			n/a <sup>3</sup>	n/a <sup>3</sup>	Α
	AM		v/c		0.4					0.0	n/a <sup>2</sup>			n/a <sup>3</sup>	n/a <sup>3</sup>	
Hwy 67 at		One-Way Stop	Delay		15.6					7.5	n/a <sup>2</sup>			n/a <sup>3</sup>	n/a <sup>3</sup>	5.7
W Walnut St.		One-way otop	LOS		С					Α	n/a <sup>2</sup>			n/a <sup>3</sup>	n/a <sup>3</sup>	Α
	PM		v/c		0.5					0.1	n/a <sup>2</sup>			n/a <sup>3</sup>	n/a <sup>3</sup>	
			Delay		19.8					7.8	n/a <sup>2</sup>			n/a <sup>3</sup>	n/a <sup>3</sup>	5.9
			LOS	n/a²	n/a <sup>2</sup>				3				Α		n/a <sup>2</sup>	Α
	AM		v/c	n/a²	n/a <sup>2</sup>				.1				0.0		n/a <sup>2</sup>	
Hwy 67/ Walnut St.		One-Way Stop	Delay	n/a <sup>2</sup>	n/a <sup>2</sup>				2.2				8.1		n/a <sup>2</sup>	1.2
at Hwy 67 / 6th St.		one may etep	LOS	n/a <sup>2</sup>	n/a <sup>2</sup>				3				Α		n/a <sup>2</sup>	Α
	PM		v/c	n/a <sup>2</sup>	n/a <sup>2</sup>				.2				0.0		n/a <sup>2</sup>	
			Delay	n/a <sup>2</sup>	n/a <sup>2</sup>				1.2				7.8		n/a²	2.0
	AM		LOS	Α	n/a <sup>2</sup>	n/a <sup>2</sup>	Α	n/a <sup>2</sup>	n/a <sup>2</sup>		С			С		Α
			v/c	0.0	n/a <sup>2</sup>	n/a <sup>2</sup>	0.0	n/a <sup>2</sup>	n/a <sup>2</sup>		0.3			0.1		
Hwy 67 / 6th St. at		Two-Way Stop	Delay	7.8	n/a <sup>2</sup>	n/a <sup>2</sup>	8.0	n/a <sup>2</sup>	n/a <sup>2</sup>		17.3			16.0		4.1
Hwy 67 / Caddo St.		, ,	LOS	Α	n/a <sup>2</sup>	n/a <sup>2</sup>	Α	n/a <sup>2</sup>	n/a <sup>2</sup>		E			С		В
	PM		v/c	0.0	n/a <sup>2</sup>	n/a <sup>2</sup>	0.0	n/a <sup>2</sup>	n/a <sup>2</sup>		0.7			0.3		
			Delay	8.0	n/a <sup>2</sup>	n/a <sup>2</sup>	8.2	n/a <sup>2</sup>	n/a <sup>2</sup>		43.9			20.9		11.0
			LOS		n/a <sup>2</sup>	n/a <sup>2</sup>	Α	n/a <sup>2</sup>			В					Α
	AM		v/c		n/a <sup>2</sup>	n/a <sup>2</sup>	0.0	n/a <sup>2</sup>			0.1					
Caddo St. at S 3rd St.		One-Way Stop	Delay		n/a <sup>2</sup>	n/a <sup>2</sup>	7.6	n/a <sup>2</sup>			11.5					1.3
S 310 St.	DM		LOS		n/a <sup>2</sup>	n/a <sup>2</sup>	A	n/a <sup>2</sup>			В					Α
	PM		v/c		n/a <sup>2</sup>	n/a <sup>2</sup>	0.0	n/a <sup>2</sup>			0.1 14.2					2.0
			Delay	۸	n/a <sup>2</sup>	n/a <sup>2</sup>	8.0	n/a <sup>2</sup>	n/a <sup>2</sup>		14.2 B			В		2.0
	AM		LOS	A 0.0	n/a <sup>2</sup>	n/a <sup>2</sup>	Α	n/a <sup>2</sup>	n/a <sup>2</sup>		0.1			0.0		Α
Cadda Ct at	AIVI		v/c	7.7	n/a n/a <sup>2</sup>	n/a n/a <sup>2</sup>	0.1 7.6	n/a n/a <sup>2</sup>	n/a n/a <sup>2</sup>		10.1			11.9		3.1
Caddo St. at S 1st St.		Signal	Delay	Α	n/a n/a <sup>2</sup>	n/a n/a <sup>2</sup>	7.6 A	n/a n/a <sup>2</sup>	n/a n/a <sup>2</sup>		В			В		3.1 A
0 100 00	PM				n/a n/a <sup>2</sup>	n/a n/a <sup>2</sup>		n/a n/a <sup>2</sup>	n/a n/a <sup>2</sup>		0.1			0.1		A
	I IVI		V/C	0.0 7.7	n/a <sup>2</sup>		0.0	n/a <sup>2</sup>	n/a <sup>2</sup>		10.2			12.4		6.5
AM		Delay	<i>1.1</i>		n/a <sup>2</sup>	7.6 C			Λ	10.2 A	٨	۸		Λ		
	ΔM		v/c	0.4	A 0.0	A 0.0	C 0.6	A 0.0	A 0.0	A 0.1	0.0	A 0.0	A 0.4	A 0.0	A 0.0	Α
Hwy 67/ Caddo St.	AIVI		Delay	18.1	0.0	0.0	20.4	0.0	0.0	4.0	0.0	0.0	0.4	0.0	0.0	10.0
at 10th St.		Signal	LOS	В	Α	Α	C C	Α	Α	4.0 A	Α	Α	0.9 A	Α	Α	В
2	PM		v/c	0.3	0.0	0.0	0.8	0.0	0.0	0.2	0.0	0.0	0.5	0.0	0.0	
	IVI		Delay	13.2	0.0	0.0	21.4	0.0	0.0	7.5	0.0	0.0	4.3	0.0	0.0	12.7
			Delay	13.2	0.0	0.0	21.4	0.0	0.0	7.5	0.0	0.0	4.3	0.0	0.0	12.1

<sup>&</sup>lt;sup>1</sup>No volume modeled making this movement <sup>2</sup>Free movement <sup>3</sup>HCM methodology does not calculate LOS due to unconventional configuration

Table 34 (Continued): Intersection Analysis – 2040 Alternative E and F – HCM

lata ma ati a n	Time Period	Control	MOE	EB	Moven	nent	WB	Mover	nent	NB	Movem	nent	SB	Movem	nent	Overall
Intersection	Time Period	Control	INIUE	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Overall
			LOS	С	Α	В	n/a 1	Α	В	Α	Α	Α	Α	Α	Α	В
	AM		v/c	0.3	0.0	0.6	n/a 1	0.0	0.4	0.1	0.0	0.1	0.4	0.0	0.0	
10th St. at Pine St.		Signal	Delay	20.7	0.0	18.9	n/a 1	0.0	17.8	0.2	0.0	0.3	6.3	0.0	0.0	10.8
		Signal	LOS	В	Α	В	В	Α	Α	Α	Α	Α	Α	Α	Α	Α
	PM		v/c	0.4	0.0	0.5	0.7	0.0	0.0	0.1	0.0	0.3	0.4	0.0	0.0	
			Delay	18.3	0.0	17.7	19.9	0.0	0.0	0.4	0.0	0.8	6.9	0.0	0.0	10.0
			LOS	n/a <sup>1</sup>	Α	В	n/a 1	Α	В	Α	Α	Α	n/a 1	Α	Α	В
	AM		v/c	n/a 1	0.0	0.8	n/a 1	0.0	0.5	0.3	0.0	0.0	n/a 1	0.0	0.1	
19th St at Dine St		Signal	Delay	n/a <sup>1</sup>	0.0	15.4	n/a 1	0.0	11.8	7.7	0.0	0.0	n/a 1	0.0	6.5	11.8
12th St. at Pine St.		Signal	LOS	В	Α	Α	В	Α	Α	Α	Α	Α	n/a 1	Α	Α	В
	PM		v/c	0.5	0.0	0.0	0.7	0.0	0.0	0.1	0.0	0.0	n/a 1	0.0	0.2	
			Delay	12.8	0.0	0.0	13.9	0.0	0.0	6.1	0.0	0.0	n/a 1	0.0	6.7	11.7

<sup>&</sup>lt;sup>1</sup>No volume modeled making this movement <sup>2</sup>Free movement

<sup>&</sup>lt;sup>3</sup>HCM methodology does not calculate LOS due to unconventional configuration

Table 35: Intersection Analysis – 2040 Alternative E and F – SimTraffic

Interpolation	Time	Control	MOE	EB	Movem	nent	WB	Moven	nent	NB	Moven	nent	SB	Movem	nent	Overall
Intersection	Period	Control	IVIOE	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Overall
	AM		LOS	n/a <sup>1</sup>		n/a 1				Α	Α			Α	Α	Α
Hwy 67 at	Alvi	One-Way	Delay	n/a <sup>1</sup>		n/a 1				2.5	1.1			2.0	2.8	1.4
Siplast Rd.	PM	Stop	LOS	Α		Α				Α	Α			Α	n/a <sup>2</sup>	Α
	1 101		Delay	5.7		2.4				4.0	1.0			3.0	n/a <sup>2</sup>	2.1
	AM		LOS	Α		Α				Α	Α			Α	Α	Α
Hwy 67 at	/ (IVI	One-Way	Delay	7.5		2.5				3.5	2.4			1.6	1.9	3.8
W Walnut St.	PM	Stop	LOS	В		Α				Α	Α			Α	Α	Α
	1 101		Delay	10.3		3.5				3.6	2.4			2.9	2.4	4.0
	AM		LOS	Α	Α			Α	Α				Α		Α	Α
Hwy 67/ Walnut St.	7 ((V)	One-Way	Delay	1.2	1.4			7.7	3.1				1.6		1.8	1.6
at Hwy 67 / 6th St.	PM	Stop	LOS	Α	Α			Α	Α				Α		Α	Α
	1 101		Delay	1.0	1.2			7.9	3.5				2.2		1.3	2.0
	AM		LOS	Α	Α	Α	Α	Α	Α	Α	Α	Α	Α	Α	n/a 1	Α
Hwy 67 / 6th St. at	7 ((V)	Two-Way	Delay	4.4	2.6	2.1	3.9	0.8	0.4	7.1	8.8	5.7	9.8	8.2	n/a 1	3.2
Hwy 67 / Caddo St.	PM	Stop	LOS	Α	Α	Α	Α	Α	Α	В	В	Α	Α	В	Α	Α
	1 101		Delay	4.8	2.8	2.0	3.2	0.7	0.4	11.3	12.1	8.0	9.7	10.2	6.2	4.4
	AM		LOS		Α	Α	Α	Α		Α		Α				Α
Caddo St. at	7 ((V)	One-Way	Delay		0.6	0.3	2.7	0.5		5.1		3.3				0.9
S 3rd St.	PM	Stop	LOS		Α	Α	Α	Α		Α		Α				Α
			Delay		0.7	0.4	3.1	0.6		7.7		5.2				1.5
	AM		LOS	Α	Α	Α	Α	Α	Α	Α	Α	Α	Α	Α	Α	Α
Caddo St. at	7	Signal	Delay	2.6	0.3	0.1	3.0	1.7	1.8	6.9	8.3	2.9	6.6	7.1	2.4	2.0
S 1st St.	PM	Olgridi	LOS	Α	Α	Α	Α	Α	Α	Α	Α	Α	Α	Α	Α	Α
			Delay	2.4	0.3	0.3	3.4	2.5	0.9	5.8	6.8	3.0	5.0	6.8	2.3	2.6
	AM		LOS	С	В	В	Α	В	Α	Α	Α	Α	Α	Α	Α	В
Hwy 67/ Caddo St.	7	Signal	Delay	22.1	18.5	12.2	9.5	18.9	9.6	4.9	4.4	1.5	6.4	5.8	3.5	10.8
at 10th St.	PM	C.g	LOS	С	В	Α	В	В	В	Α	Α	Α	Α	Α	Α	В
			Delay	22.0	18.0	8.8	19.1	19.9	11.9	6.5	5.9	2.1	8.5	8.1	5.6	12.2
	AM		LOS	С	С	В	n/a 1	В	Α	В	Α	Α	Α	Α	Α	В
10th St. at Pine St.	,	Signal	Delay	25.4	22.0	12.5	n/a <sup>1</sup>	17.2	6.5	10.1	3.0	0.4	7.2	6.8	4.5	10.9
	PM	2.3	LOS	С	В	В	В	В	В	В	Α	Α	Α	Α	Α	В
			Delay	27.4	18.8	12.3	19.7	16.9	10.2	13.6	5.4	2.6	9.2	8.9	6.4	11.9
	AM		LOS	n/a 1	В	Α	n/a 1	В	Α	n/a <sup>2</sup>	Α	Α	n/a 1	Α	Α	Α
AM 12th St. at Pine St.	Signal	Delay	n/a <sup>1</sup>	13.6	8.1	n/a 1	10.6	5.8	n/a <sup>2</sup>	6.9	4.2	n/a 1	5.6	3.2	9.5	
	PM	2.3	LOS	В	В	Α	В	В	Α	n/a <sup>2</sup>	Α	Α	n/a 1	Α	Α	В
			Delay	15.2	11.9	7.8	10.8	12.9	7.6	n/a <sup>2</sup>	6.9	3.9	n/a 1	7.1	3.8	10.7

<sup>&</sup>lt;sup>1</sup>No Volume modeled making this movement

<sup>&</sup>lt;sup>2</sup>Low volume modeled making this movement

Table 36: Intersection Analysis – 2040 Alternative G – HCM

Intersection	Time Period	Control	MOE	EB	Moven	nent	WB	Mover	nent	NB	Moven	nent	SB	Movem	nent	Overall
IIILETSECTION	IIIIIe Fellou	COTILIOI	WIOL	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Overall
			LOS		n/a <sup>1</sup>					Α	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	Α
	AM		v/c		n/a <sup>1</sup>					0.0	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	
Hwy 67 at		One-Way Stop	Delay		n/a <sup>1</sup>					7.4	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	0.0
Siplast Rd.		One way clop	LOS		В					Α	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	Α
	PM		v/c		0.0					0.0	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	
			Delay		11.4					7.8	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	0.2
			LOS		С					Α	n/a <sup>2</sup>			n/a <sup>3</sup>	n/a <sup>3</sup>	Α
	AM		v/c		0.4					0.0	n/a <sup>2</sup>			n/a <sup>3</sup>	n/a <sup>3</sup>	
Hwy 67 at		One-Way Stop	Delay		15.6					7.5	n/a <sup>2</sup>			n/a <sup>3</sup>	n/a <sup>3</sup>	5.7
W Walnut St.		· , p	LOS		С					Α	n/a <sup>2</sup>			n/a <sup>3</sup>	n/a <sup>3</sup>	Α
	PM		v/c		0.5					0.1	n/a <sup>2</sup>			n/a <sup>3</sup>	n/a <sup>3</sup>	
			Delay	•	19.8					7.8	n/a <sup>2</sup>			n/a <sup>3</sup>	n/a <sup>3</sup>	5.9
			LOS	n/a <sup>2</sup>	n/a <sup>2</sup>				3				Α		n/a <sup>2</sup>	Α
	AM		v/c	n/a <sup>2</sup>	n/a <sup>2</sup>				.1				0.0		n/a <sup>2</sup>	
Hwy 67/ Walnut St.		One-Way Stop	Delay	n/a <sup>2</sup>	n/a <sup>2</sup>				2.2				8.1		n/a <sup>2</sup>	1.2
at Hwy 67 / 6th St.		, ,	LOS	n/a <sup>2</sup>	n/a <sup>2</sup>				3				Α		n/a <sup>2</sup>	Α
	PM		v/c	n/a <sup>2</sup>	n/a <sup>2</sup>				.2				0.0		n/a <sup>2</sup>	
			Delay	n/a <sup>2</sup>	n/a <sup>2</sup>	2			1.2				7.8		n/a <sup>2</sup>	2.0
	AM		LOS	Α	n/a <sup>2</sup>	n/a <sup>2</sup>	Α	n/a <sup>2</sup>	n/a <sup>2</sup>		С			С		Α
			v/c	0.0	n/a <sup>2</sup>	n/a <sup>2</sup>	0.0	n/a <sup>2</sup>	n/a <sup>2</sup>		0.3			0.1		
Hwy 67 / 6th St. at		Two-Way Stop	Delay	7.8	n/a <sup>2</sup>	n/a <sup>2</sup>	8.0	n/a <sup>2</sup>	n/a <sup>2</sup>		17.3			16.0		4.1
Hwy 67 / Caddo St.	514	, ,	LOS	Α	n/a <sup>2</sup>	n/a <sup>2</sup>	Α	n/a <sup>2</sup>	n/a <sup>2</sup>		E 0.7			С		В
	PM		v/c	0.0	n/a <sup>2</sup>	n/a <sup>2</sup>	0.0	n/a <sup>2</sup>	n/a <sup>2</sup>		0.7			0.3		44.0
			Delay	8.0	n/a <sup>2</sup>	n/a <sup>2</sup>	8.2	n/a <sup>2</sup>	n/a <sup>2</sup>		43.9			20.9		11.0
	***		LOS		n/a <sup>2</sup>	n/a <sup>2</sup>	A	n/a <sup>2</sup>			В					Α
	AM		v/c		n/a <sup>2</sup>	n/a <sup>2</sup>	0.0	n/a <sup>2</sup>			0.1					4.0
Caddo St. at S 3rd St.		One-Way Stop	Delay		n/a <sup>2</sup>	n/a <sup>2</sup>	7.6	n/a <sup>2</sup>			11.5					1.3
S 310 St.	DM		LOS		n/a <sup>2</sup>	n/a <sup>2</sup>	A	n/a <sup>2</sup>			В					Α
	PM		v/c		n/a <sup>2</sup>	n/a <sup>2</sup>	0.0	n/a <sup>2</sup>			0.1					0.0
			Delay	Δ.	n/a <sup>2</sup>	n/a <sup>2</sup>	8.0	n/a <sup>2</sup>	, 2		14.2 B			D		2.0
	A N A		LOS	Α	n/a <sup>2</sup>	n/a <sup>2</sup>	A	n/a <sup>2</sup>	n/a <sup>2</sup>					В		Α
0 11 01 1	AM		v/c	0.1	n/a <sup>2</sup>	n/a <sup>2</sup>	0.0	n/a <sup>2</sup>	n/a <sup>2</sup>		0.0 10.6			0.0		4.0
Caddo St. at S 1st St.		Signal	Delay	7.7	n/a <sup>2</sup>	n/a <sup>2</sup>	7.5	n/a <sup>2</sup>	n/a <sup>2</sup>		10.0 B					1.6
O 190 OL	PM		LOS	A	n/a	n/a	A	n/a <sup>2</sup>	n/a <sup>2</sup>					B 0.1		Α
	FIVI		v/c	0.0	n/a <sup>2</sup>	n/a <sup>2</sup>	0.0		n/a <sup>2</sup>		0.0			11.0		2.0
			Delay	7.7	n/a <sup>2</sup>	n/a <sup>2</sup>	7.5	n/a <sup>2</sup>	n/a <sup>2</sup>	Δ.	10.9	Δ.	۸		۸	2.0
	AM		LOS	B	Α	Α	C	Α	A	A	Α	Α	Α 0.4	Α	Α	Α
Lhua, 67/ O-44- O	AIVI		v/c	0.4	0.0	0.0	0.6	0.0	0.0	0.1	0.0	0.0	0.4	0.0	0.0	10.0
Hwy 67/ Caddo St. at 10th St.		Signal	Delay	18.1	0.0	0.0	20.4 C	0.0	0.0	4.0	0.0	0.0	0.9	0.0	0.0	10.0 B
at 10th Ot.	PM		LOS	В	Α	A		Α	Α	A	Α	Α	A	Α	Α	В
	FIVI		v/c	0.3	0.0	0.0	0.8	0.0	0.0	0.2	0.0	0.0	0.5	0.0	0.0	10.7
			Delay	13.2	0.0	0.0	21.4	0.0	0.0	7.5	0.0	0.0	4.3	0.0	0.0	12.7

<sup>&</sup>lt;sup>1</sup>No volume modeled making this movement <sup>2</sup>Free movement <sup>3</sup>HCM methodology does not calculate LOS due to unconventional configuration

Table 36 (Continued): Intersection Analysis – 2040 Alternative G – HCM

lata ra a atia a	Time Period	Control	MOE	EB	Movem	nent	WB	Mover	nent	NB Movement			SB	Overall		
Intersection	IIIIle Peliod	Control	INIOE	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
			LOS	С	Α	В	n/a 1	Α	В	Α	Α	Α	Α	Α	Α	В
	AM		v/c	0.3	0.0	0.6	n/a 1	0.0	0.4	0.1	0.0	0.1	0.4	0.0	0.0	
10th St. at Pine St.		Signal	Delay	20.7	0.0	18.9	n/a 1	0.0	17.8	0.2	0.0	0.3	6.3	0.0	0.0	10.8
TOUT OL ALT THE OL		Signal	LOS	В	Α	В	В	Α	Α	Α	Α	Α	Α	Α	Α	Α
	PM		v/c	0.4	0.0	0.5	0.7	0.0	0.0	0.1	0.0	0.3	0.4	0.0	0.0	
	PM		Delay	18.3	0.0	17.7	19.9	0.0	0.0	0.4	0.0	8.0	6.9	0.0	0.0	10.0
			LOS	n/a 1	Α	В	n/a 1	Α	В	Α	Α	Α	n/a 1	Α	Α	В
	AM		v/c	n/a 1	0.0	0.8	n/a 1	0.0	0.5	0.3	0.0	0.0	n/a 1	0.0	0.1	
12th St. at Pine St.		Signal	Delay	n/a 1	0.0	15.4	n/a 1	0.0	11.8	7.7	0.0	0.0	n/a 1	0.0	6.5	11.8
12th St. at Pine St.	Signal	LOS	В	Α	Α	В	Α	Α	Α	Α	Α	n/a 1	Α	Α	В	
	PM		v/c	0.5	0.0	0.0	0.7	0.0	0.0	0.1	0.0	0.0	n/a 1	0.0	0.2	
	PM		Delay	12.8	0.0	0.0	13.9	0.0	0.0	6.1	0.0	0.0	n/a 1	0.0	6.7	11.7

<sup>&</sup>lt;sup>1</sup>No volume modeled making this movement <sup>2</sup>Free movement

<sup>&</sup>lt;sup>3</sup>HCM methodology does not calculate LOS due to unconventional configuration

Table 37: Intersection Analysis – 2040 Alternative G – SimTraffic

late ve e et e v	Time	Control	МОГ	EB	Movem	ent	WB	Moven	nent	NB	Moven	nent	SB	Moven	nent	Overell
Intersection	Period	Control	MOE	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Overall
	AM		LOS	n/a 1		n/a 1				n/a <sup>2</sup>	Α			Α	n/a <sup>2</sup>	Α
Hwy 67 at	Alvi	One-Way	Delay	n/a 1		n/a 1				n/a <sup>2</sup>	1.1			2.2	n/a <sup>2</sup>	1.5
Siplast Rd.	PM	Stop	LOS	n/a <sup>2</sup>		n/a <sup>2</sup>				n/a <sup>2</sup>	Α			Α	n/a <sup>2</sup>	Α
	1 101		Delay	n/a <sup>2</sup>		n/a <sup>2</sup>				n/a <sup>2</sup>	0.9			2.4	n/a <sup>2</sup>	1.8
	AM		LOS	Α		Α				Α	Α			Α	Α	Α
Hwy 67 at	7	One-Way	Delay	7.5		2.4				3.5	2.6			1.7	2.1	3.9
W Walnut St.	PM	Stop	LOS	Α		Α				Α	Α			Α	Α	Α
			Delay	9.1		2.6				3.7	2.6			2.4	2.3	3.9
	AM		LOS	Α	Α			Α	Α				n/a <sup>2</sup>		Α	Α
Hwy 67/ Walnut St.		One-Way	Delay	0.4	0.2			5.6	2.6				n/a²		0.1	0.8
at Hwy 67 / 6th St.	PM	Stop	LOS	Α	Α			Α	Α				Α		Α	Α
			Delay	1.1	1.2			7.5	3.4				1.1		1.2	2.0
	AM		LOS	Α	Α	Α	Α	Α	Α	Α	Α	Α	n/a <sup>2</sup>	Α	n/a 1	Α
Hwy 67 / 6th St. at		Two-Way	Delay	4.0	2.6	2.4	4.0	0.6	0.5	7.5	8.3	6.1	n/a <sup>2</sup>	8.4	n/a 1	3.1
Hwy 67 / Caddo St.	PM	Stop	LOS	Α	Α	Α	Α	Α	Α	В	В	Α	Α	Α	Α	Α
			Delay	4.9	2.6	2.1	4.5	0.5	0.2	11.4	14.2	8.5	7.3	9.1	3.5	4.4
	AM		LOS		Α	Α	Α	Α		Α		Α				Α
Caddo St. at		One-Way	Delay		0.3	0.5	2.4	0.4		5.3		3.1				0.9
S 3rd St.	PM	Stop	LOS		Α	Α	Α	Α		Α		Α				Α
			Delay		0.5	0.4	2.5	0.5		5.3		5.2				1.2
	AM		LOS	Α	Α	Α	n/a 1	Α	Α	Α	Α	n/a 1	Α	Α	Α	Α
Caddo St.		Signal	Delay	0.0	0.2	0.2	n/a 1	0.5	0.8	5.3	5.3	n/a 1	5.0	4.9	3.2	1.3
at S 1st St.	PM		LOS	Α	Α	Α	n/a 1	Α	Α	n/a <sup>2</sup>	Α	n/a 1	Α	Α	Α	Α
			Delay	2.3	0.4	0.5	n/a 1	1.0	0.5	n/a <sup>2</sup>	10.0	n/a 1	5.5	7.2	2.6	1.5
	AM		LOS	В	В	Α	n/a <sup>2</sup>	В	A	n/a <sup>2</sup>	A	n/a <sup>2</sup>	A	Α	A	В
Hwy 67/ Caddo St. at 10th St.		Signal	Delay	12.0	19.0	9.4	n/a <sup>2</sup>	18.8	9.2	n/a <sup>2</sup>	5.1	n/a <sup>2</sup>	6.2	6.4	4.2	10.8
at ion st.	PM		LOS	C	C	A	n/a <sup>2</sup>	C	B	A	A	A	A	A	A	B
			Delay	31.8	20.1	9.3	n/a <sup>2</sup>	21.3	12.4	6.0	6.4	0.7	9.5	8.9	4.2	13.1
	AM		LOS	B	В	B	n/a 1	В	A	В	A	n/a <sup>2</sup>	Α	A	A	B
10th St. at Pine St.		Signal	Delay	18.4	18.0	13.5	n/a 1	19.0	8.7	10.8	4.3	n/a <sup>2</sup>	10.0	6.3	4.4	10.4
	PM		LOS	C	B	В	n/a <sup>2</sup>	B	B	В	Α	A	В	A	A	B
			Delay	25.1	17.0	11.5	n/a <sup>2</sup>	15.1	11.2	14.0	5.0	2.2	11.5	9.5	6.6	11.5
	AM		LOS	n/a 1	B	A 7.4	n/a 1	A	Α	n/a <sup>2</sup>	A	B	n/a 1	Α	A	Α
12th St. at Pine St.		Signal	Delay	n/a 1	14.7	7.4	n/a 1	10.0	4.3	n/a <sup>2</sup>	6.0	11.0	n/a 1	5.1	3.3	9.4
	PM		LOS	n/a <sup>2</sup>	B	Α	n/a <sup>2</sup>	B	Α 7.4	n/a <sup>2</sup>	A	A	n/a 1	A	Α	A
			Delay	n/a <sup>2</sup>	11.5	6.4	n/a <sup>2</sup>	10.7	7.4	n/a <sup>2</sup>	6.6	1.1	n/a 1	6.6	3.7	9.6

<sup>&</sup>lt;sup>1</sup>No Volume modeled making this movement

<sup>&</sup>lt;sup>2</sup>Low volume modeled making this movement

#### 2040 Intersection Analysis, Highway 67 Improvement Alternative Condition

In addition to analyzing bypass alternatives, a Highway 67 improvement alternative (Alternative D) was also evaluated. The improved alignment extends north from west of the intersection of Highway 67 and Walnut Street toward the intersection of Caddo Street with 10<sup>th</sup> Street. Future traffic volumes for the Highway 67 Improvement Alternative are shown in **Figure 16.** The intersection analyses are shown in **Table 38** through **Table 39**. The overall intersection LOS for the study area intersections on the eastern end of study area operate acceptably during AM and PM peak hours with Alternative D improvements. The complete results are provided in **Appendix B - Intersection Operational Analysis Results.** 

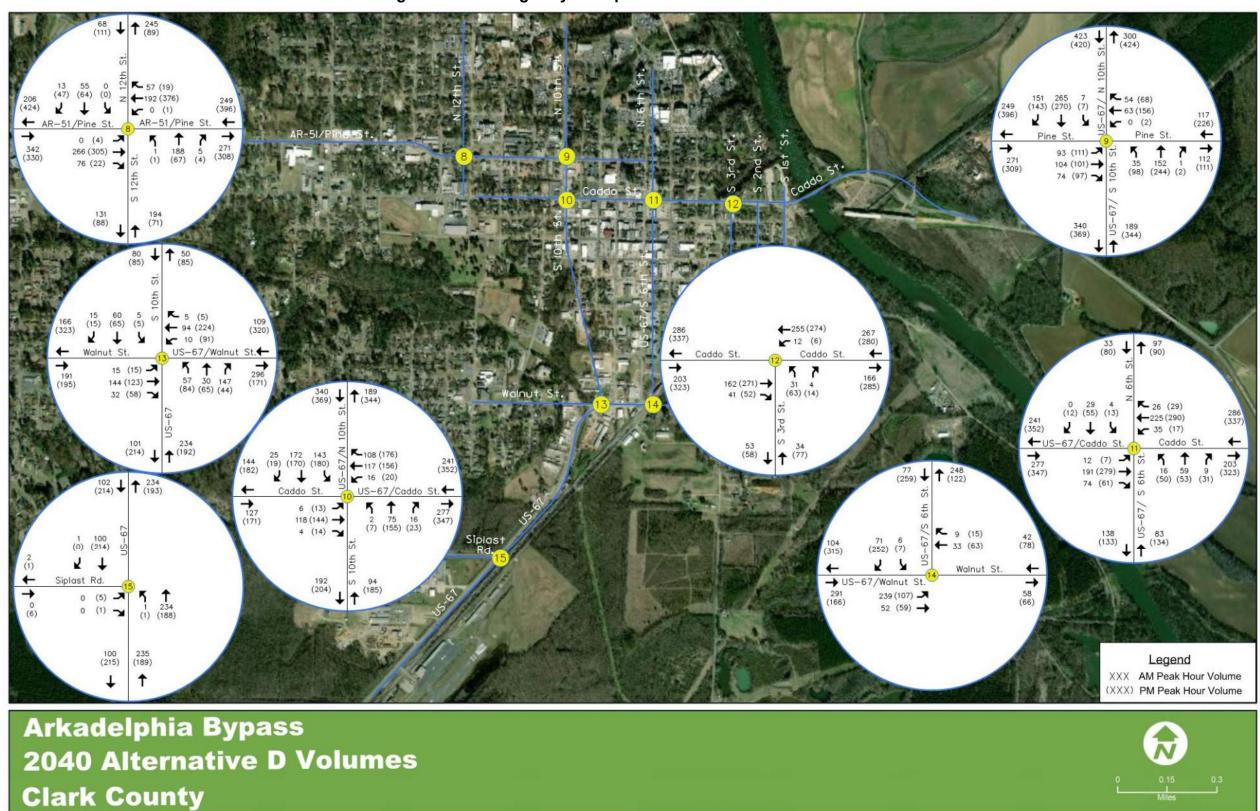


Figure 16: 2040 Highway 67 Improvement – Alternative D Volumes

Table 38: Intersection Analysis – 2040 Alternative D – HCM

	Time			EB	Movem	ent	WE	3 Movem	nent	NB	Moveme	ent	SB	Movem	ent	
Intersection	Time Period	Control	MOE	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Overall
			LOS		n/a 1					Α	n/a²			n/a <sup>2</sup>	n/a <sup>2</sup>	Α
	AM		v/c		n/a 1					0.0	n/a²			n/a <sup>2</sup>	n/a <sup>2</sup>	
Hwy 67 at		One-Way	Delay		n/a 1					7.5	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	0.0
Siplast Rd.		Stop	LOS		В					Α	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	Α
	PM		v/c		0.0					0.0	n/a²			n/a <sup>2</sup>	n/a <sup>2</sup>	
			Delay		11.6					7.8	n/a <sup>2</sup>			n/a <sup>2</sup>	n/a <sup>2</sup>	0.2
			LOS		С			В			Α			Α		В
	AM		v/c		0.6			0.4			0.3			0.1		
Hwy 67 at		Signal	Delay		21.1			19.6			4.4			3.3		11.6
W Walnut St.		0.9	LOS		В			В			Α			Α		В
	PM		v/c		0.4			0.7			0.3			0.1		40.5
			Delay	n/a <sup>2</sup>	14.8 n/a <sup>2</sup>			17.9	3		7.7		۸	6.5	n/a <sup>2</sup>	13.5 A
	AM		v/c	n/a <sup>2</sup>	n/a <sup>2</sup>				.1				A 0.0		n/a <sup>2</sup>	A
1 h C7/ Waland Ob. at		O W		n/a <sup>-</sup>	n/a <sup>-</sup>				. i I.6				8.0		n/a <sup>2</sup>	1.2
Hwy 67/ Walnut St. at Hwy 67 / 6th St.		One-Way Stop	Delay LOS	n/a <sup>-</sup>	n/a <sup>-</sup>				3				ο.υ		n/a <sup>-</sup>	1.3 A
Tiwy 07 / Out St.	PM	оюр	v/c	n/a <sup>2</sup>	n/a <sup>2</sup>				.2				0.0		n/a <sup>2</sup>	А
	PIVI			n/a <sup>-</sup>	n/a <sup>2</sup>				.z 2.7				7.7		n/a -	2.1
			Delay LOS	n/a-	n/a <sup>-</sup>	n/a <sup>2</sup>	Α	n/a <sup>2</sup>	n/a <sup>2</sup>		С		1.1	С	n/a²	2.1 A
	AM		v/c	0.0	n/a <sup>2</sup>	n/a <sup>2</sup>	0.0	n/a <sup>2</sup>	n/a <sup>2</sup>		0.3			0.1		A
1 h C7 / C4h C4 -4	Alvi	T \\/		7.9	n/a n/a <sup>2</sup>	n/a n/a <sup>2</sup>	8.1	n/a n/a <sup>2</sup>	n/a n/a <sup>2</sup>		19.9			18.3		3.9
Hwy 67 / 6th St. at Hwy 67 / Caddo St.		Two-Way Stop	Delay	7.9 A	n/a <sup>-</sup>	n/a <sup>-</sup>	ο.1	n/a <sup>-</sup>	n/a <sup>-</sup>		19.9 D			C		3.9 A
Tiwy of / Caudo St.	PM	Stop	v/c	0.0	n/a <sup>2</sup>	n/a <sup>2</sup>	0.0	n/a <sup>2</sup>	n/a <sup>2</sup>		0.6			0.3		А
	FIVI		Delay	8.1	n/a n/a <sup>2</sup>	n/a n/a <sup>2</sup>	8.2	n/a n/a <sup>2</sup>	n/a n/a <sup>2</sup>		34.2			24.1		7.5
			LOS	0.1	n/a <sup>-</sup>	n/a <sup>-</sup>	0.2 A	n/a <sup>-</sup>	n/a-		34.2 B			24.1		7.5 A
	AM		v/c		n/a n/a <sup>2</sup>	n/a n/a <sup>2</sup>	0.0	n/a n/a <sup>2</sup>			0.1					A
Caddo St. at	Alvi	One-Way	Delay		n/a n/a <sup>2</sup>	n/a n/a <sup>2</sup>	7.8	n/a n/a <sup>2</sup>			13.0					1.1
S 3rd St.		Stop	LOS		n/a n/a <sup>2</sup>	n/a n/a <sup>2</sup>	7.0 A	n/a n/a <sup>2</sup>			C					Α
o old ot.	PM	оюр	v/c		n/a n/a <sup>2</sup>	n/a n/a <sup>2</sup>	0.0	n/a n/a <sup>2</sup>			0.2					A
	FIVI				n/a n/a <sup>2</sup>	n/a n/a <sup>2</sup>	8.2	n/a n/a <sup>2</sup>			16.2					1.9
			Delay	D			-		Λ	۸		۸	۸	٨	٨	
	AM		LOS v/c	B 0.3	A 0.0	A 0.0	B 0.7	A 0.0	A 0.0	A 0.1	A 0.0	A 0.0	A 0.4	A 0.0	A 0.0	Α
Hwy 67/ Caddo St. at			V/C Delay	16.3	0.0	0.0	19.5	0.0	0.0	5.0	0.0	0.0	1.2	0.0	0.0	9.5
		Signal	,													
1001 00	PM															В
	1 141															11 9
10th St.	PM	Signal	LOS v/c Delay	B 0.4 13.4	A 0.0 0.0	A 0.0 0.0	C 0.8 20.7	A 0.0 0.0	A 0.0 0.0	A 0.3 8.1	A 0.0 0.0	A 0.0 0.0	A 0.6 4.7	A 0.0 0.0	A 0.0 0.0	B 11.9

<sup>&</sup>lt;sup>1</sup>No volume modeled making this movement <sup>2</sup>Free movement <sup>3</sup>HCM methodology does not calculate LOS due to unconventional configuration

Table 38 (Continued): Intersection Analysis – 2040 Alternative D – HCM

	T:			EB	Movem	ent	WE	Movem	ent	NB	Moveme	ent	SB	ent		
Intersection	Time Period	Control	MOE	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Overall
			LOS	С	Α	С	n/a 1	Α	В	Α	Α	Α	Α	Α	Α	В
	AM		v/c	0.4	0.0	0.6	n/a 1	0.0	0.4	0.1	0.0	0.2	0.5	0.0	0.0	
10th St. at Pine St.		Signal	Delay	20.4	0.0	18.3	n/a 1	0.0	17.2	0.2	0.0	0.4	8.1	0.0	0.0	10.7
Tour Ot. at 1 mo Ot.		Olgilai	LOS	В	Α	В	В	Α	Α	Α	Α	Α	Α	Α	Α	В
	PM		v/c	0.4	0.0	0.7	0.6	0.0	0.0	0.2	0.0	0.3	0.5	0.0	0.0	
			Delay	18.5	0.0	19.0	19.3	0.0	0.0	0.7	0.0	8.0	8.2	0.0	0.0	10.7
			LOS	n/a 1	Α	В	n/a <sup>1</sup>	Α	В	Α	Α	Α	n/a 1	Α	Α	В
	AM		v/c	n/a 1	0.0	0.8	n/a 1	0.0	0.6	0.3	0.0	0.0	n/a 1	0.0	0.1	
12th St. at Pine St.		Signal	Delay	n/a 1	0.0	15.8	n/a 1	0.0	12.8	8.0	0.0	0.0	n/a 1	0.0	6.8	12.4
12th St. at Pine St.		Signai	LOS	В	Α	Α	В	Α	Α	Α	Α	Α	n/a 1	Α	Α	В
	PM		v/c	0.6	0.0	0.0	0.7	0.0	0.0	0.1	0.0	0.0	n/a 1	0.0	0.2	
			Delay	12.5	0.0	0.0	13.9	0.0	0.0	7.7	0.0	0.0	n/a 1	0.0	8.4	12.2

<sup>&</sup>lt;sup>1</sup>No volume modeled making this movement

<sup>&</sup>lt;sup>2</sup>Free movement <sup>3</sup>HCM methodology does not calculate LOS due to unconventional configuration

Table 39: Intersection Analysis – 2040 Alternative D – SimTraffic

	:			EB	Movem	ent	WE	Movem	nent	NE	B Moveme	ent	SB	Movem	ent	
Intersection	Time Period	Control	MOE	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Overall
	AM		LOS	n/a 1		n/a 1				,	4			ı	4	Α
Hwy 67 at	Aivi	One-Way	Delay	n/a 1		n/a 1				0	.9			2	.2	1.3
Siplast Rd.	PM	Stop	LOS	Α		Α				ı	4			ı	4	Α
	I IVI		Delay	2.6		1.0				0	.6			1	.7	1.2
	AM		LOS	В	С	В	В	В	Α	Α	Α	Α	Α	Α	Α	В
Hwy 67 at	\(\text{INI}\)	Signal	Delay	19.5	20.9	11.2	11.3	17.3	5.9	6.5	5.3	4.6	7.3	5.8	3.2	11.0
W Walnut St.	PM	Olgital	LOS	В	В	Α	В	В	В	В	Α	Α	В	Α	Α	В
	F IVI		Delay	19.0	13.6	7.1	17.4	17.4	15.4	10.2	9.2	4.1	10.9	9.0	5.0	13.0
	AM		LOS	Α	Α			Α	Α				A		Α	Α
Hwy 67/ Walnut St. at		One-Way	Delay	0.5	0.9			6.6	2.9				4.6		2.2	1.4
Hwy 67 / 6th St.	PM	Stop	LOS Delay	A 0.4	A 0.6			A 8.2	A 3.7				A 2.8		A 1.7	A 2.4
			LOS	Α	Α	Α	Α	Α.2	Α	В	В	Α	Α	Α	n/a 1	Α
Hwy 67 / 6th St. at	AM	Two-Way	Delay	3.4	2.2	1.7	3.1	1.8	0.8	11.0	10.5	5.1	5.5	8.1	n/a 1	3.4
Hwy 67 / Caddo St.		Stop	LOS	A	A	A	A	A	A	С	В	A	В	В	В	A
,	PM		Delay	8.9	2.4	2.2	3.0	1.7	1.1	15.7	13.3	6.6	10.2	14.3	12.8	4.4
			LOS	0.0	Α	A	A	A		A		A			.2.0	Α
Caddo St. at	AM	One-Way	Delay		0.9	0.6	4.6	2.2		5.3		2.8				1.9
S 3rd St.		Stop	LOS		Α	A	Α	A		Α		Α				Α
	PM	·	Delay		1.4	1.1	4.8	2.3		7.2		5.3				2.4
			LOS	В	В	Α	С	С	В	n/a²	Α	Α	Α	Α	Α	В
Hwy 67/ Caddo St. at	AM		Delav	11.5	14.6	3.3	22.0	21.7	13.2	n/a <sup>2</sup>	3.3	1.7	7.0	5.4	3.3	10.5
10th St.		Signal	LOS	С	В	A	В	С	В	A	Α	Α	В	В	В	В
	PM		Delay	30.7	17.3	9.1	19.9	22.0	15.6	6.1	5.8	3.8	12.8	10.8	11.0	14.2
			LOS	С	С	В	n/a 1	В	Α	В	Α	n/a <sup>2</sup>	В	Α	Α	В
1011 01 101	AM		Delay	26.3	22.5	11.6	n/a 1	17.2	6.7	15.6	3.3	n/a <sup>2</sup>	10.7	8.8	5.8	11.4
10th St. at Pine St.	D14	Signal	LOS	С	В	В	n/a <sup>2</sup>	В	Α	В	Α	A	В	В	Α	В
	PM		Delay	25.6	18.3	13.5	n/a <sup>2</sup>	15.1	7.9	18.8	6.5	4.2	13.2	12.2	8.5	13.2
			LOS	n/a 1	В	Α	n/a 1	Α	Α	n/a <sup>2</sup>	Α	Α	n/a 1	Α	Α	Α
4011 OL 1 D	AM	0: 1	Delay	n/a 1	11.5	8.6	n/a 1	9.5	5.6	n/a <sup>2</sup>	9.0	3.6	n/a 1	7.4	3.2	9.3
12th St. at Pine St.	DM	Signal	LOS	В	В	Α	n/a <sup>2</sup>	В	Α	Α	Α	Α	n/a 1	В	Α	В
	PM		Delay	17.1	12.6	7.5	n/a <sup>2</sup>	12.8	7.8	1.8	8.3	1.1	n/a 1	10.7	6.9	11.6

<sup>&</sup>lt;sup>1</sup>No Volume modeled making this movement

<sup>&</sup>lt;sup>2</sup>Low volume modeled making this movement

#### **2040 Roadway Segment Analysis, Alternative Conditions**

The roadway segment of the proposed bypass was also analyzed using the *HCS* two-lane highway module. Based on the volume data, a peak hour factor of 0.82 was utilized for the AM peak and a peak hour factor of 0.86 was utilized for the PM peak. From the previous study, the Alternative 3 and Alternative 7 bypass was projected to carry 4% heavy vehicles. This same percentage was used for in this study.

For the existing interchange bypass alternative analysis, the bypass was broken into four segments. Segment 1, between Pine Street and Walnut Street, was estimate to be approximately 0.4 miles in length with a base free flow speed for 45 mph based on the posted speed limit plus 10 mph according to *HCM* methodology. Between Walnut Street and near 13<sup>th</sup> Street, Bypass Segment 2 was estimated to be approximately 1.5 miles long with a base free flow speed of 55 mph. The third segment, between 13<sup>th</sup> Street and Clay Street, was estimated to be approximately 2.0 miles long with a base free flow speed of 55 mph. Segment 4, between Clay Street and Caddo Street/Highway 51 was estimated to be 0.2 miles long with a base free flow speed of 45 mph.

For the new interchange bypass alternative analysis, the bypass was broken into three segments. Segment 1, between the new interchange and 13th Street, was estimate to be approximately 2.3 miles long with a base free flow speed of 55 mph based on the posted speed limit plus 10 mph according to *HCM* methodology. The second segment, between 13<sup>th</sup> Street and Clay Street, was estimated to be approximately 2.0 miles long with a base free flow speed of 55 mph. Segment 3, between Clay Street and Caddo Street/Highway 51 was estimated to be 0.2 miles long with a base free flow speed of 45 mph.

The results of this analysis are shown in **Table 40** below. The complete results from this analysis are provided in **Appendix C - Roadway Segment Operational Analysis Results**.

These results demonstrate that the corridor currently operates at acceptable LOS throughout the study area.

Table 40: Roadway Segment Analysis – 2040 Alternative Action Conditions – HCM

Existing Interchanage Bypass Alternative													
		Two-Lane			AM					PM			
Segment	Direction	Hwy Class	LOS	ATS	PTSF	PFFS	v/c	LOS	ATS	PTSF	PFFS	v/c	
		,	LUS	(mph)	(%)	(%)	V/C	LUJ	(mph)	(%)	(%)	V/C	
Bypass - Pine Street to Walnut Street		Class III	С	33.9	65.6	79.7	0.22	С	33.9	62.7	79.7	0.21	
Bypass - Walnut Street to 13th Street	FB	Class I	В	50.9	12.1	96.0	0.06	В	50.7	28.2	95.6	0.06	
Bypass - 13th Street to Clay Street	LD	Class I	В	51.4	21.8	96.9	0.03	В	51.0	22.0	96.2	0.03	
Bypass - Clay Street to Hwy 51		Class III	Α	39.0	31.2	91.8	0.03	В	38.9	31.9	91.6	0.03	
Bypass - Hwy 51 to Clay Street		Class III	Α	39.0	35.3	91.8	0.03	В	38.9	35.8	91.6	0.04	
Bypass - Clay Street to 13th Street	WB	Class I	В	51.4	24.7	96.9	0.30	В	51.3	25.2	96.8	0.04	
Bypass - 13th Street to Walnut Street	VVD	Class I	В	50.8	11.2	95.8	0.06	В	50.7	29.4	95.7	0.06	
Bypass - Walnut Street to Pine Street		Class III	С	34.0	62.9	80.0	0.20	С	33.6	67.5	79.1	0.23	
	New Interc	hanage Byp	pass Alternative										
		Two-Lane		AM				PM					
Segment	Direction	Hwy Class	LOS	ATS	ATS PTSF		v/c	LOS	ATS	PTSF	PFFS	v/c	
		nwy Ciass	LUS	(mph)	(%)	(%)	V/C	LUS	(mph)	(%)	(%)	V/C	
Bypass - New interchange to 13th Street		Class I	В	50.9	12.1	96.0	0.06	В	50.7	28.2	95.6	0.06	
Bypass - 13th Street to Clay Street	EB	Class I	В	51.4	21.8	96.9	0.03	В	51.0	22.0	96.2	0.03	
Bypass - Clay Street to Hwy 51		Class III	Α	39.0	31.2	91.8	0.03	В	38.9	31.9	91.6	0.03	
Bypass - Hwy 51 to Clay Street		Class III	Α	39.0	35.3	91.8	0.03	В	38.9	35.8	91.6	0.04	
Bypass - Clay Street to 13th Street	WB	Class I	В	51.4	24.7	96.9	0.30	В	51.3	25.2	96.8	0.04	
Bypass - 13th Street to New Interchange		Class I	В	50.8	11.2	95.8	0.06	В	50.7	29.4	95.7	0.06	

# **Appendix B** — Public Involvement Synopsis

# Public Involvement Synopsis Public Meeting

#### **Public Involvement Synopsis**

ARDOT Job Number 070442 Hwy. 67 – Hwy. 51 (Arkadelphia Bypass) Clark County Tuesday, February 5, 2019

An open-forum Public Involvement Meeting for the proposed project was held at Henderson State University (Garrison Center Grand Ball Room), 1045 McKnight Drive in Arkadelphia from 4:00 – 7:00 p.m. on Tuesday, February 5, 2019. Special efforts to involve minorities and the public in the meeting included the following:

- Display advertisement placed in The Arkansas Democrat Gazette on Sunday, January 20 and Sunday, February 3, 2019
- Public Service Announcement (PSA) ran on Power 92.3FM from Saturday, February 2 through Tuesday, February 5, 2019
- Outreach letters mailed and/or emailed to public officials, stakeholders, and local churches
- Flyers distributed in the project area
- Information posted on multiple websites and social media platforms
- News release published

The following information was available for inspection and comment:

- An Environmental Constraints Map exhibit on aerial photograph at a scale of 1 inch equals 0.25 miles
- An Environmental and Safety Constraints table with potential impacts for each alignment or alternative
- Two Corridor Map roll plots on aerial photography showing seven proposed bypass alignments at a scale of 1 inch equals 200 feet
- Two sets of four Interchange Alternative roll plots on aerial photography showing interchanges on Highway 51 near I-30 at a scale of 1 inch equals 200 feet
- A 5-minute rotating PowerPoint with voiceover that provided an overview of the project and the public meeting format

Handouts for the public included a comment form, a summary sheet, and small-scale maps illustrating the corridor alignments and interchange alternatives, which were identical to the aerial photograph displays. Copies of the handouts, exhibits, and PowerPoint slides are attached.

Page 1 of 7 Updated: Tuesday, April 30, 2019



#### **Public Meeting**

Table 1 describes the results of the participation at the public officials meeting and public meeting.

Table 1	
Public Participation	Totals
Attendance at public officials meeting (including ARDOT and Garver staff	19
Attendance at public meeting	145
Total attendance	164
Comment forms received	52
Letters received (no comment form)	5

An analysis of the responses received as a result of the public form survey is shown in Table 2.

Table 2	
Survey Form Results (52 forms)	Totals
Feel there is a need for a proposed bypass	46
Do not feel there is a need for a proposed bypass	5
Prefer improvements to Highway 67/51 instead	15
Do not prefer improvements to Highway 67/51 instead	35
Knowledge of historical, archeological or cemetery sites	0
Knowledge of area environmental constraints	8
Beneficial impacts due to the proposed project	22
Adverse impacts due to the proposed project	9

- In general, a majority of those who commented expressed a need for a bypass and did not prefer improvements to Highway 67/51 instead.
- Most commenters felt the bypass would have beneficial impacts.

Page 2 of 7 Updated: Tuesday, April 30, 2019



#### **Public Meeting**

An analysis of the responses received as a result of the public survey for the bypass alignments and interchange alternatives is shown in Table 3.

Table 3		
Survey Results	Support	Oppose
West: Alignment A	27	10
West: Alignment B	24	9
West: Alignment C	10	17
East: Alignment D	29	12
East: Alignment E	10	16
East: Alignment F	13	16
East: Alignment G	20	13
	T.	1
Interchange Alternative 1	12	17
Interchange Alternative 2	15	11
Interchange Alternative 3	11	17
Interchange Alternative 4	13	14

- Western Alignments A and B received a majority of support, while Alignment C received the greatest opposition.
- To the east, Alignments D and G received the most support, while Alignments E and F received more opposition than support.
- Interchange Alternative 2 was the only alternative that received more support than opposition.

Garver staff reviewed all comments received and evaluated their contents. The summary of comments listed below reflects the personal perception or opinion of the person or organization make the statement. The sequencing of the comments is random and is not intended to reflect importance or numerical values. Some of the comments were combined and/or paraphrased to simplify the synopsis process.

The following is a listing of comments concerning issues associated with this project:

Page 3 of 7 Updated: Tuesday, April 30, 2019



#### **Public Meeting**

#### **General Bypass Comments**

- Nearly all comments expressed support for a bypass south of Arkadelphia and said there is a need. Commenters said truck traffic is currently an issue in the Central Business District and the number of trucks is expected to increase if the Sun Paper mill is constructed.
- Commenters discussed issues that trucks have maneuvering through the Central Business District, as well as intersections that need to be looked at. Areas mentioned included turning movements from Highway 51 to Highway 67, at I-30 exit 123, and at 10<sup>th</sup> Street and Pine Street, 10<sup>th</sup> Street and Caddo Street, and 6<sup>th</sup> Street and Caddo Street. One commenter said trucks drive up on sidewalks to make turns.
- Commenters said the bypass will open up opportunities for new business, economic
  development, and growth, as well as improve access and decrease congestion within the
  CBD, as well as safety. One comment said to make the bypass four lanes with a turning
  lane.
- Several commenters requested the bypass be farther outside of town.
- One commenter wished information was available regarding the number of trucks coming from each corridor.

#### **Specific Alignment Comments**

- Alignment A Commenters said it will provide opportunity for economic development and make it easier for trucks to maneuver and not come through the city.
- Alignment B Commenters liked the use of Red Hill Road, with some suggesting the bypass continue farther south down Red Hill Road. Commenters felt using Red Hill Road would lessen impact on business and residential areas. Others mentioned future hotel construction along the route.
- Alignment C Several commenters said the alignment is too close to residential areas.
- Alignment D Commenters in support said this route is cost effective, should be completed regardless of the bypass location, will make it easy for trucks to maneuver north and south and alleviates truck traffic from the CBD. Commenters with concerns said the alignment comes too far into town and trucks will still be in the CBD. Others said to make sure not to cut off pedestrian access due to this route and don't want quality of life hindered in the surrounding neighborhoods. A few suggested extending 10th Street as well as softening the turn at 10th Street and Caddo Street.
- Alignments E and F There were only a couple comments about these alignments. One
  person thought the routs were too messy. Another said it would be nice to avoid the
  business area.
- Alignment G Commenters said the river bridge on this alignment should have been
  constructed instead of the new bridge on Caddo Street, and others felt it would now be a
  waste of money to build a second bridge. Others, whether in support or opposed, were
  concerned about the cost of a new bridge. Commenters also felt that using Alignment G
  with Alignment A/B would be the easiest route for trucks and eliminate 90-degree turns.

Page 4 of 7 Updated: Tuesday, April 30, 2019



#### **Public Meeting**

#### **General Interchange Alternative Comments**

- At least four commenters said they do not want the project to include roundabouts. Some commenters seemed to think there would be multiple roundabouts in this location.
- Several commenters said they wanted to see the mobile home park moved.
- One person felt the alternatives are too complex and need to be simplified.

#### **Specific Interchange Alternative Comments**

- Only a few commenters specifically mentioned an interchange in their comments. One commenter said Interchanges 1 and 4 allow space for traffic.
- Regarding Interchange 2, commenters said it has the least impact on business/residential areas and they liked the alignment with Professional Park Drive.

#### **Suggested Routes and other Suggestions**

Commenters provided numerous suggestions on new alignments or improving upon the proposed alignments. These suggestions included the following:

- Use Red Hill Road to connect to Highway 67. Won't cross rock quarry hill.
- Move I-30 interchange to Gum Springs exit or Snyder Road overpass (MM 71).
- Improve the existing CBD repave and add turn signals at 10th and Caddo; 10th and Pine.
- Lift log-truck restrictions on interstate from Exits 69 to 78 and allow to carry full tonnage.
- Move I-30 eastbound exit farther south on Red Hill Road before Marriott. This will decrease congestion at Professional Park on-ramp and eliminate need for a roundabout at that location.
- Connect Hwy. 67 from Caddo Valley to Hwy. 51, cross Hwy. 7, continue to Gum Springs and I-30.
- Make 6th and 7th streets one-way.
- One resident is glad an option that impacted his home on Walnut (from 2013 report) is no longer under consideration, but wants to be notified if reinstated. That route followed west end of Walnut Street from Pine Street south until Walnut turns east. Then continued south and east to meet Highway 67 north of Siplast plant.
- Take route east on Pine until it intersects with 10th Street, and then turn south to Hwy. 67.
- Route should be Hwy. 67 to Country Club Road/Pizza Hut and proceed west across I-30 to Mt. Zion Road and then to Reynolds.
- I-30 at Country Club Road should have ingress and egress to highway.
- Reroute Alignment G south of the rail road bridge and continue where Alignment E is planned.
- Follow along Pine and Route 51 on Caddo. Schools are moving out of the High School.

Page 5 of 7 Updated: Tuesday, April 30, 2019



#### **Public Meeting**

- Take Red Hill Road farther south and take former road south of Alignment B to connect to Alignment A. This will avoid hills and reduce erosion issues.
- Connect 51 to Gum Springs bridge over I-30 by going south on the west side of I-30
- Take a route south of the airport.
- Create a road on the west side of the interstate coming in on Hwy 51/26 from the west.
   Make the road turn between the old highway department building and Chicken Express that goes south to the overpass at Gum Springs exit.
- One person wondered why it took so long to notify landowners who might be impacted of various options under consideration after first phase of study in 2013.

#### **Environmental or Historical**

- Two commenters mentioned a landfill / old dump in area of Alignment G behind ballpark.
- Four commenters mentioned wetlands in area of Alignments A / B / C.
- Three commenters mentioned tectum dump site north of Alignment A and C, south of Walnut Street, west of Hwy. 67.
- Two commenters believe an EIS should be prepared.

#### **Adverse Property Impacts**

- The Red River Baptist Association is next to proposed bridge.
- Owner Cox Mobile Manor said some interchange alternatives will impact / ruin her property.
- Owner on 9th Street said Alignment D impacts property, shop, house, etc. but is willing to sell
- Resident on Walnut Street said bypass comes close to western edge of his property (17 acres). Believes it misses his property and doesn't object to this option. Wants to make sure recreational cabin is not impacted.
- Alignment D routes highway adjacent to owner's property, increasing traffic noise.
- Three commenters referenced the same family property, which is located along Alignments A and C, with two submitting the same form and one submitting a three-page letter.
  - The identical forms stated they thought the 2013 report showed a bypass was not warranted. At the public meeting, information was not provided on environmental and financial effects. Social information was shown but not as a take-home document. Should have been available under FOI as public record. Asked if the new bridge is a waste of tax monies and poor planning if a new bridge is built. There should be a route utilizing all existing roadways. Any of the plans will forcibly take land in their family for generations.
  - The three-page letter said a bypass should be based on relevant, repeatable and factual data, not speculation of a future pulp mill being constructed. The commenter referenced the 2013 bypass study and 2010 Highway Capacity Manual and said LOS within Arkadelphia does not warrant a new bypass. The commenter said the destination survey from that study indicates vehicles crossing the bridge are

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#### **Public Meeting**

destined to the area of Arkadelphia, not to I-30 and west of Arkadelphia. A new bypass serves less of a purpose with Pine Street being widened will help east-west travel time and better handle future volumes. The commenter said alternative alignments were presented without supportive information as to why the project is needed (Purpose and Need) and associated costs. The commenter said Alignments A, B, or C from an environmental consideration will cause destruction, loss, and or degradation to wetlands and streams. The commenter opposes Alignments A, B, and C and the associated interchange alternatives. The commenter agreed that reducing truck volumes in the CBD could be beneficial, and if this is the primary objective, Alignments E or F should be considered before Alignment D, with a preference for Alignment F because it impacts fewer residents, businesses and road crossings. The commenter said either Alignment A or C will impact family land passed down for generations, and that the land is enrolled in the US Department of Agriculture's Conservation Stewardship Program.

#### Attachments:

- Public handouts, including blank comment form
- Small-scale display copy

Page 7 of 7 Updated: Tuesday, April 30, 2019



# ARKANSAS DEPARTMENT OF TRANSPORTATION (ARDOT) CITIZEN COMMENT FORM

ARDOT JOB NUMBER 070442 Hwy. 67 – Hwy. 51 (ARKADELPHIA BYPASS) CLARK COUNTY

#### LOCATION:

Henderson State University (Garrison Center Grand Ball Room) 1045 McKnight Drive, Arkadelphia, AR 4:00 – 7:00 p.m., Tuesday, February 5, 2019

Make your comments on this form and leave it with ARDOT or Garver personnel at the meeting or mail it by 4:30 p.m. on Wednesday, February 20, 2019 to: Jon Hetzel, Garver, 4701 Northshore Drive, North Little Rock, AR 72118. Alternatively, send the form via email to PublicInvolvement@GarverUSA.com.

Ciliali lo	ubliciti	voivementa	Jaiveluun.	COIII.
Yes				d for a proposed bypass on the southern side of S. Highway 67 and State Highway 51?
	_			
	_			
	_			
Hwy. 51 support o	intercha or oppos	inge alternativ	es near I-3 ments or ar	or oppose the corridor alignments and 0 by circling your choices below. You may ny combinations of alignments. Please provide
Corridor	Alignn	nents		
Align A	Align ✓ <u>×</u>	B Align C	Align D  ✓	
Align E ✓	Align <b></b> ✓	F Align G ✓		
×	×	×		
Intercha	nge Alt	ernatives		
Alt 1 ✓	Alt 2	Alt 3	Alt 4 ✓	
×	×	×	×	

(Continued on Back)

	Arkadelphia Central Business District instead of a bypass alignment?
No	Do you know of any historical sites, family cemeteries, or archaeological sites in the project area? Please note and discuss with staff.
No	Do you know of any environmental constraints, such as endangered species, hazardous waste sites, or existing or former landfills, in the vicinity of the project? Please note and discuss with staff.
No	Do you have a suggestion that would make this proposed project better serve the needs of the community?
	Do you feel that the proposed project will have any impacts (  Beneficial or  Adverse) on your property and/or community (economic, environmental, social, etc.)? Please explain.
e a pr	cessary for the ARDOT to contact property owners along potential routes. It operty owner along or adjacent to the route under consideration, please nation below. Thank you.
(Pleas	e <i>Print</i> ):
	Phone: ()
	<del></del>
	No No No Carrotte information (Please Si:

For additional information, please visit our website at <a href="www.ardot.gov">www.ardot.gov</a>.

# Arkadelphia Bypass Hwy. 67 – Hwy. 51 Connection Improvement Study Job 070442



# What is the Arkadelphia Bypass Project?

The Arkansas Department of Transportation (ARDOT) is continuing to build on past planning studies and evaluating alternatives to improve traffic flow through Arkadelphia, particularly through the Central Business District (CBD). A primary concern is the truck traffic carrying raw lumber materials that must negotiate several difficult turns and pass through the CBD.

# What are the Alternatives Being Considered?

#### **Bypass Alignments**

Seven alignments provide a proposed bypass south of Arkadelphia.

- Alignments A-C connect Hwy. 51 (near I-30) to Hwy. 67 near the airport.
- Alignments D-G connect Hwy. 67 to Hwy. 51 near the Ouachita River. Routes use either the recently completed river bridge or construct one farther south.

#### Interchange Alternatives

Four interchange alternatives connect the bypass at Hwy. 51 near I-30.

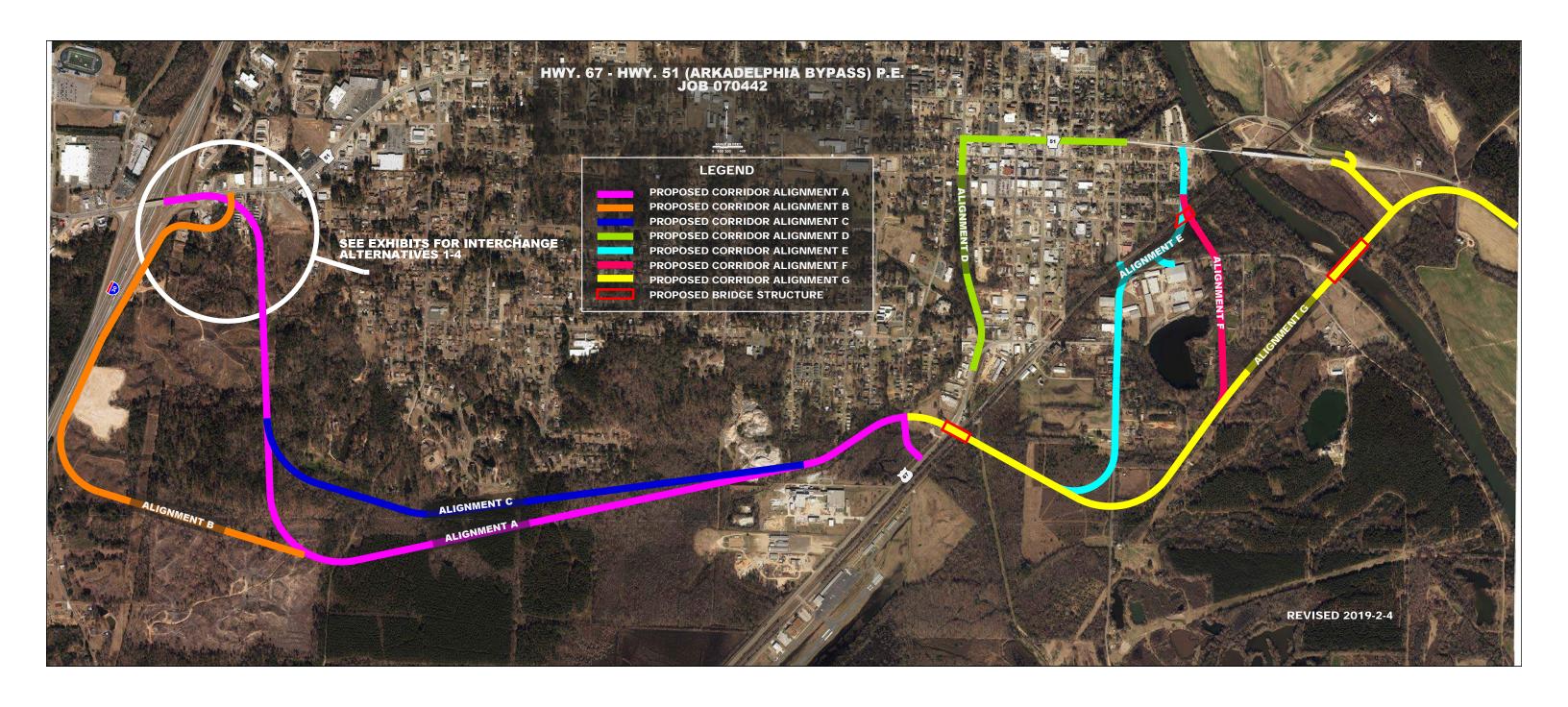
# Why are we Meeting today?

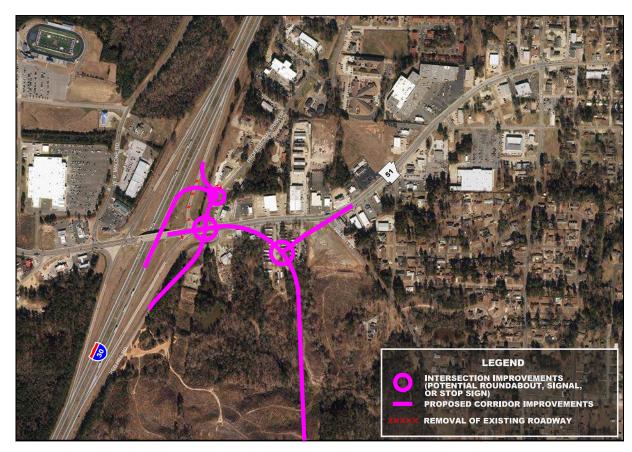
The format for the meeting is an Open House, which means there is no formal presentation. You are encouraged to view the preliminary alternatives, ask questions, and provide comments to our staff. Your comments are very important and could help shape the location and nature of future improvements. Comments may be submitted through Wednesday, February 20, 2019.

#### What Comes Next?

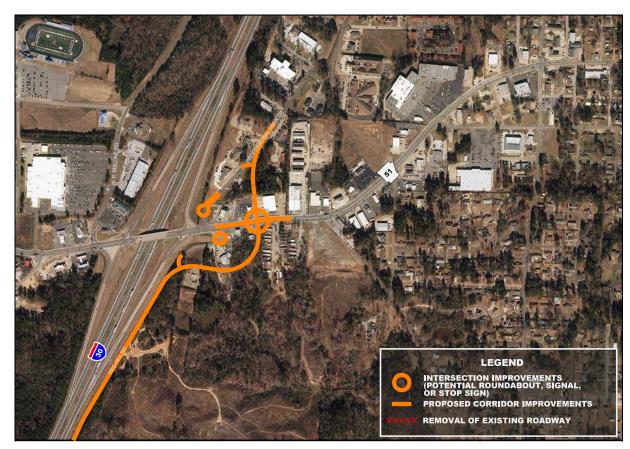
Comments received today and over the next two weeks will be considered. Based on the comments, along with environmental and design considerations, revisions will be made and a preferred alternative selected. Another meeting will be held to request feedback on the preferred alternative. There is not a funding source for construction at this time.

Meeting information is available at: http://ardot.gov/public\_meetings/2019\_PM/070442/070442.aspx

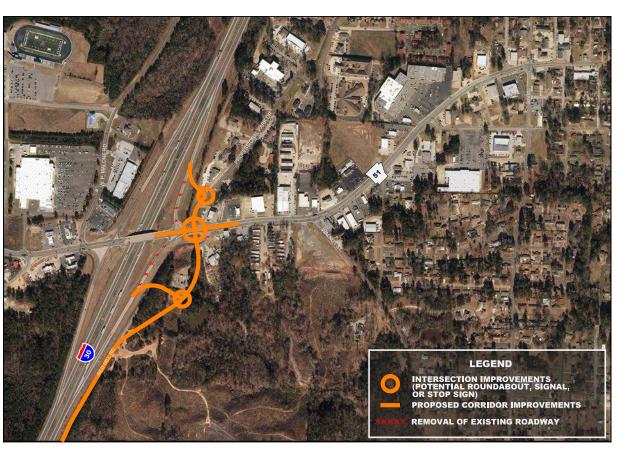




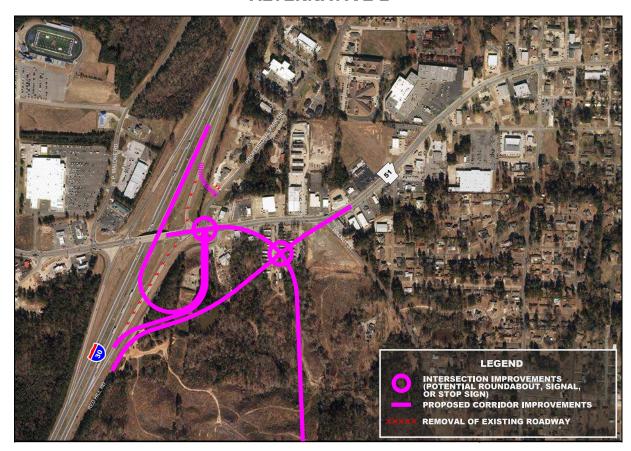
I-30/Hwy. 51 Interchange ALTERNATIVE 1



I-30/Hwy. 51 Interchange ALTERNATIVE 3

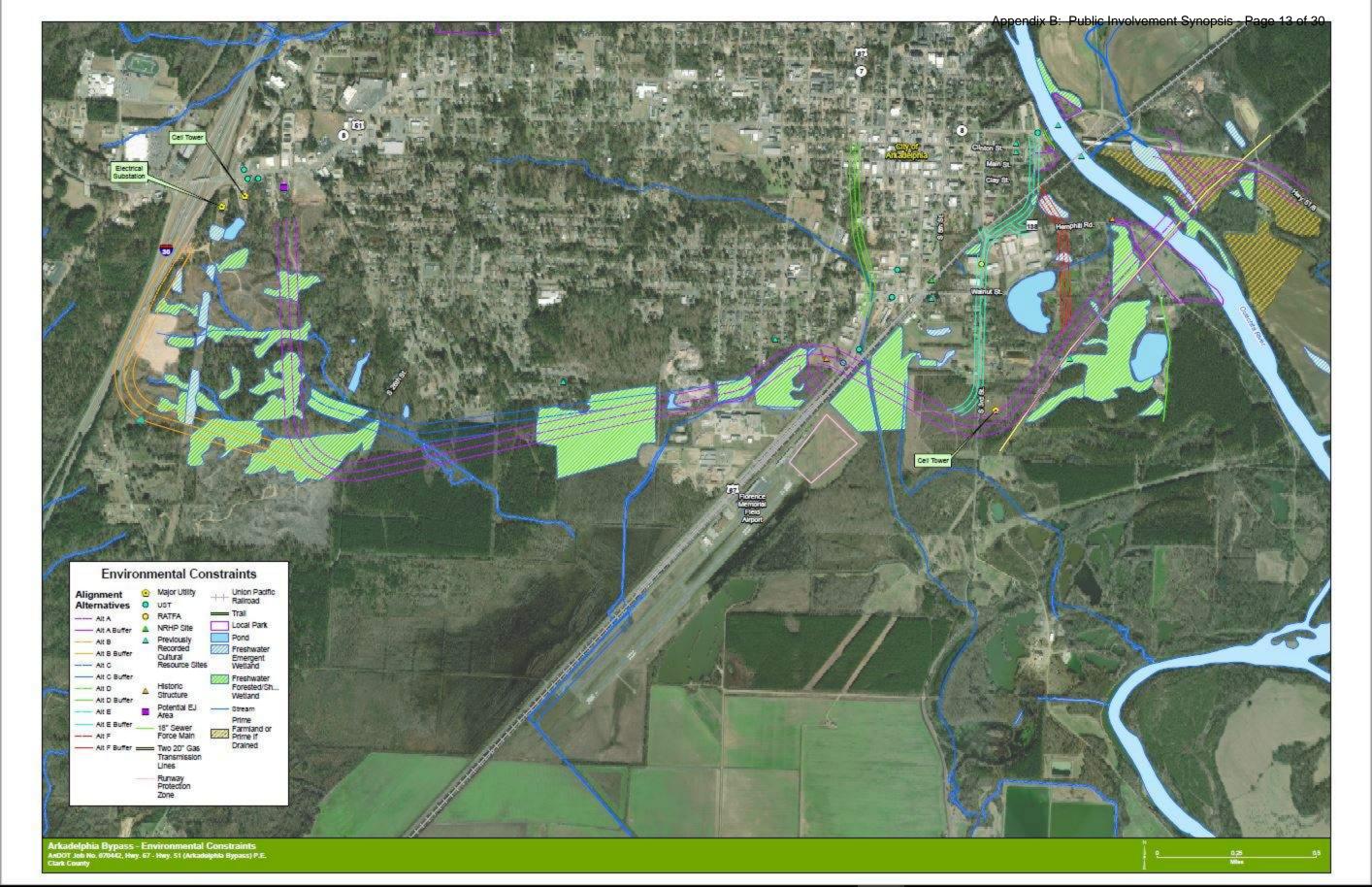


I-30/Hwy. 51 Interchange ALTERNATIVE 2



I-30/Hwy. 51 Interchange ALTERNATIVE 4



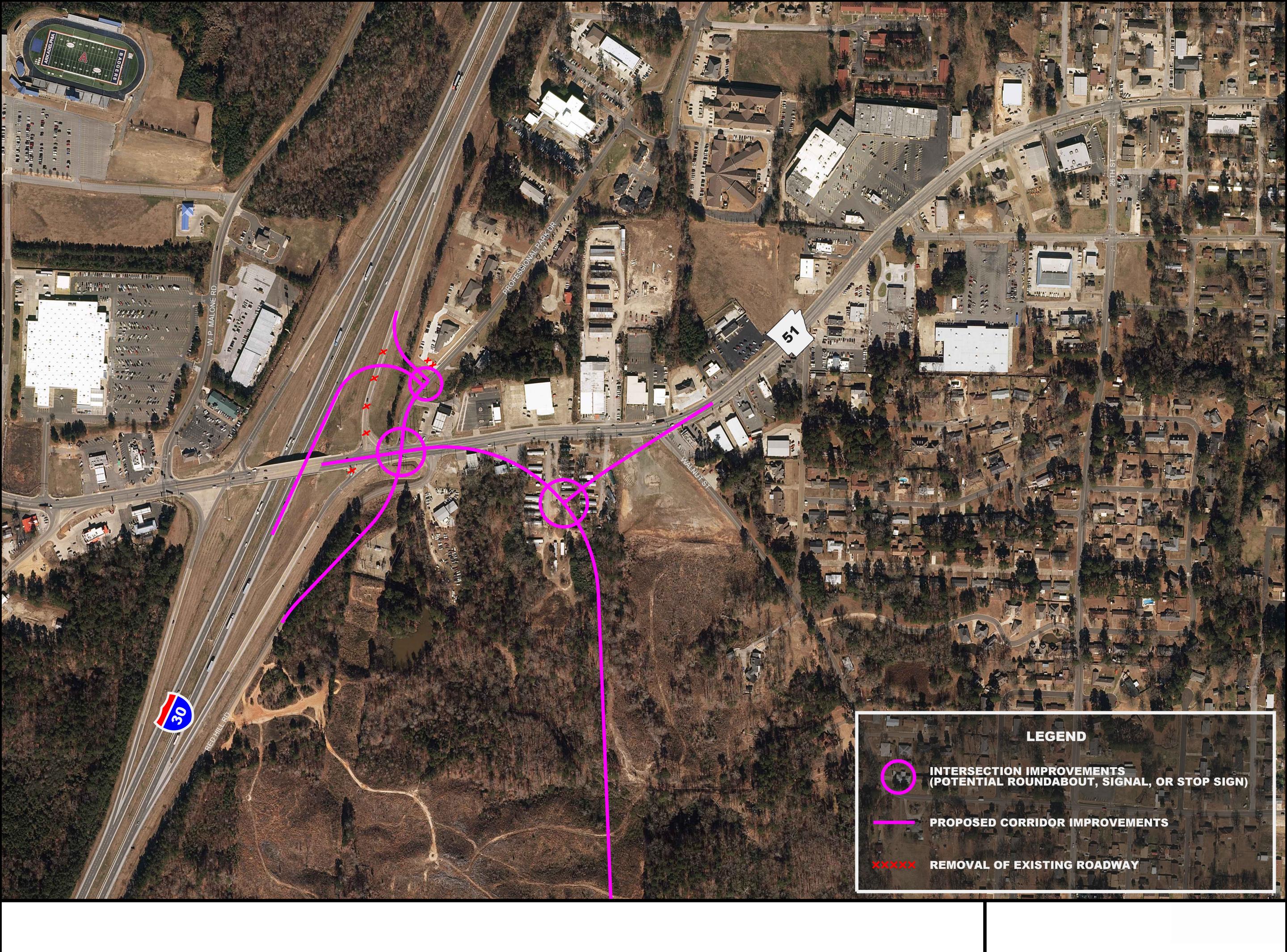


Arkadelphia Bypass
ARDOT No. 070442, Hwy. 67 - Hwy. 51 (Arkadelphia Bypass) P.E.

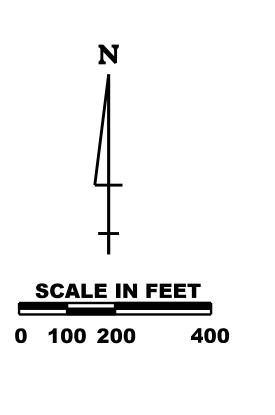
## **Environmental and Safety Constraints - All Options**

			Potential Impacts												
Item #	Resource Category	Description	Mgnta/Yllw Alt A&G	Orange Alt B	Blue Alt C	Green Alt D	Cyan Alt E	Red Alt F	Interchange Alt 1	Interchange Alt 2	Interchange Alt 3	Interchange Alt 4			
1	Alternative Length	Miles	5.1	1.4	1.0	0.5	1.1	0.4							
2	Residential Impacts	Relocations	5	0	0	5	11	3	27	0	27	27			
3	Other Structures or Major Utilities	Relocations	4	1	0	4	6	4	0	0	3	3			
4	Business Impacts	Relocations	2	0	0	10	7	0	3	0	5	5			
5	Roads	Crossings	8	1	0	4	8	2	3	2	3	3			
6	Railroads	Crossings	1	0	0	0	1	1	0	0	0	0			
7	Section 4(f) & 6(f)	Number	1	0	0	0	1	0	0	0	0	0			
8	Federal Endangered and Threatened Species	Known Occurrences	1	0	0	0	0	0	0	0	0	0			
9	Floodplains/Floodway	Acres	84	0	9.0	3.8	16.3	16.5	0	0	0	0			
10	Hazardous Materials	Known Sites	4	0	0	0	1	0	2	2	3	3			
11	Historic Resources	Known Sites	2	0	0	0	0	0	0	0	0	0			
12	Streams	Crossings	8	1	1	1	1	1	0	0	0	0			
13	Wetlands	Acres	50.6	12.1	13.4	0	0.5	1.8	0	0.5	0	1			



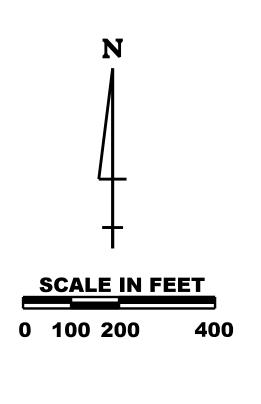


# I-30/HWY. 51 INTERCHANGE ALTERNATIVE 1



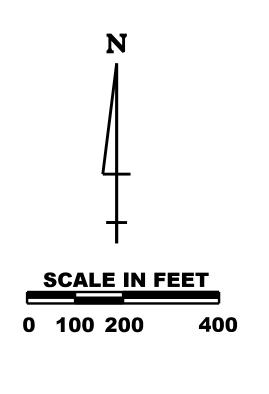


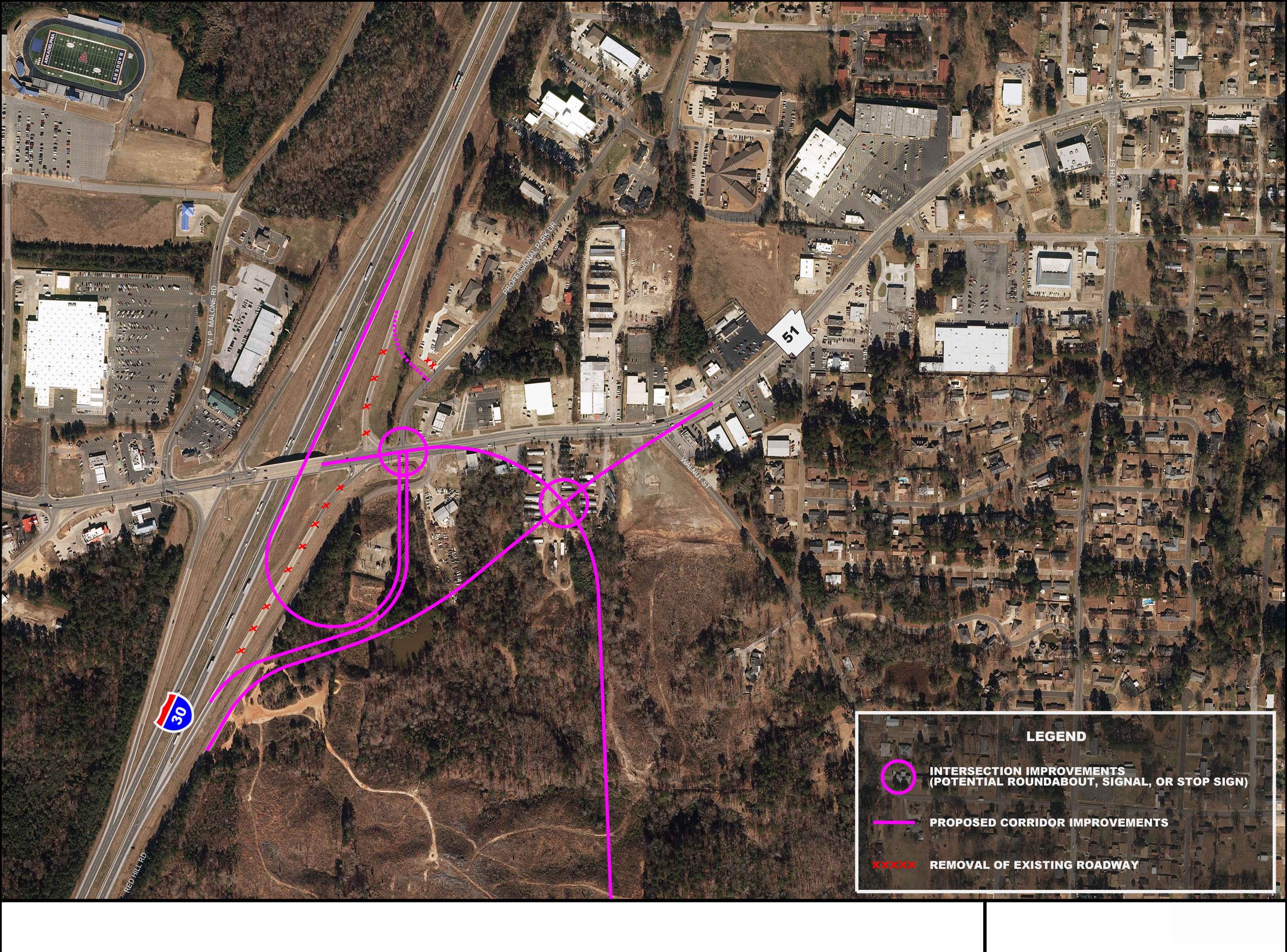
# I-30/HWY. 51 INTERCHANGE ALTERNATIVE 2



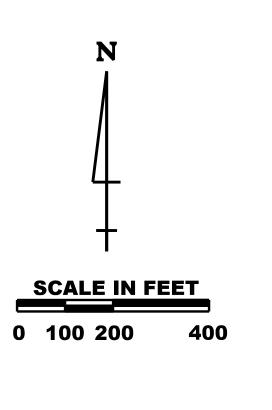


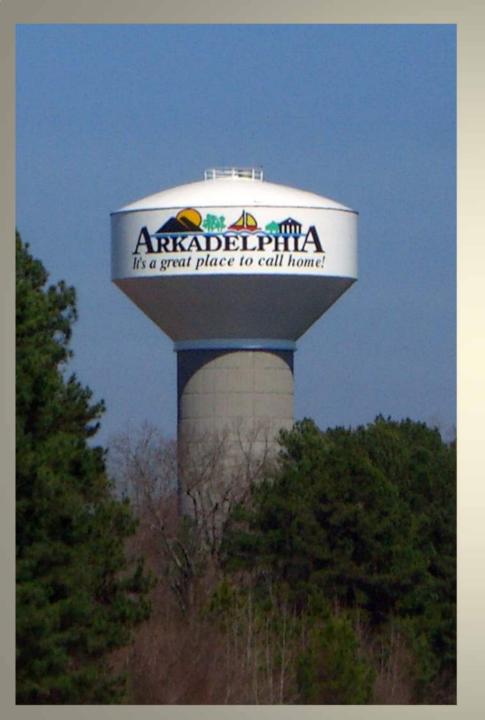
# I-30/HWY. 51 INTERCHANGE ALTERNATIVE 3





# I-30/HWY. 51 INTERCHANGE ALTERNATIVE 4





# WELCOME

Public Meeting for the Arkadelphia Bypass Clark County

**February 5, 2019** 

## **OPEN HOUSE FORMAT**

- The following 5 minute presentation gives a brief project summary and how you can participate in this meeting.
- This is an "open house," so please review the maps and other materials at your own pace. You may stay and ask questions and review the materials as long as you need between 4 and 7 p.m.
- Our staff are wearing name tags. Please Ask Questions.
- Please give us feedback by <u>filling out a comment form.</u>

# WHAT IS THE NEED FOR THIS PROJECT?

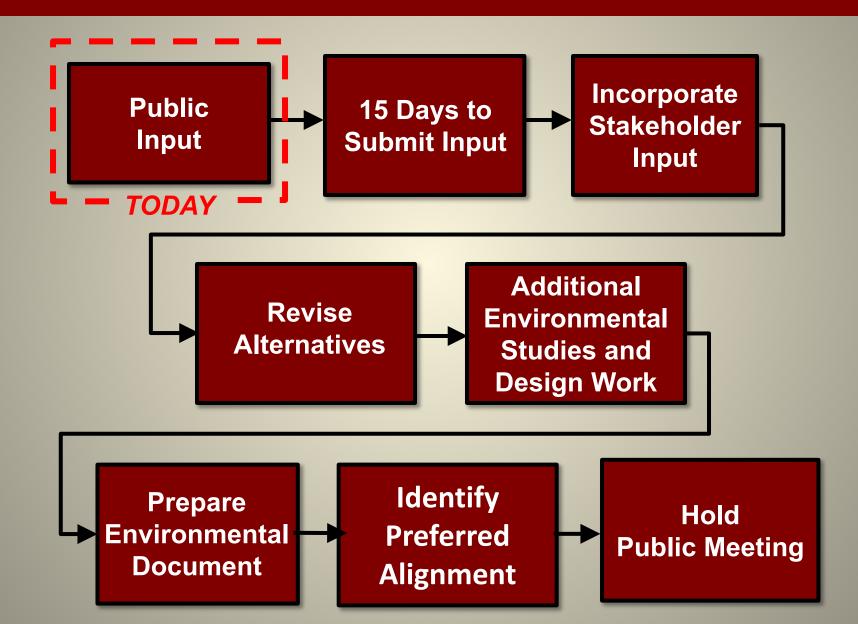
Traffic through Arkadelphia includes of a large number of trucks carrying raw lumber materials. These trucks must negotiate tight turns, especially in the Central Business District (CBD), creating mobility and safety issues.

The PURPOSE of this study is to improve traffic flow through Arkadelphia, particularly through the Central Business District, by providing improvements that enhance traffic operations and result in a safer road for vehicles and pedestrians.

# **WHY ARE WE HERE TONIGHT?**

- This project has federal funding, which requires a review process called an Environmental Assessment (EA).
- The purpose of the EA is to identify a Preferred Alignment that minimizes the negative impacts to the natural and social environment while addressing the needs of the project to the greatest extent possible.
- We are holding this meeting tonight because <u>Participation</u> by the public and local officials is <u>key to the success of</u> <u>this EA.</u>
- The EA study process began in April 2018 and is estimated to be complete in November 2019.

# WHAT HAPPENS NEXT?



# WHAT IS CONSIDERED IN THE EA PROCESS?

# Some of the considerations in the project development are:

- Road Crossings
- Bridges and Culverts
- Railroad Crossings
- Residential Impacts
- Business Impacts
- Economic Impacts
- Indirect Impacts
- Environmental Justice
- Historic and Archeological

- Wetlands
- Floodplains
- Endangered Species
- Safety
- Community Connections
- Topography (impacts design and costs)
- Hazardous Materials Sites
- Right of Way and Construction Costs

# WHAT ARE WE REVIEWING?

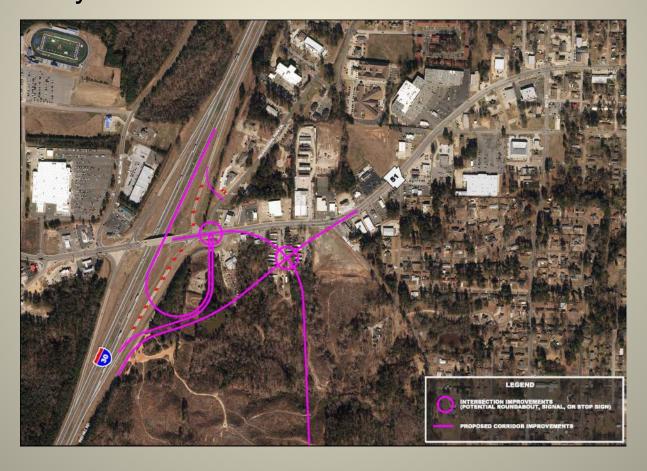
Preliminary alignments have been developed with consideration for various environmental and design concerns. There are exhibits on the tables for you to review the various bypass options.

Detailed design has not been completed – the alignments presented are close approximations so that environmental and social impacts may be evaluated and discussed.



## WHAT ARE WE REVIEWING?

In addition to seven options for the main bypass segments, there are four interchange options where the western end of the bypass connects to Hwy. 51 near I-30.



## WHAT DO WE EXPECT FROM THIS MEETING?

Our ultimate goal for this EA process is to identify roadway improvements that minimize the negative impacts to the natural and social environment while improving mobility, safety and the quality of life for Arkadelphia's residents and those traveling in your city.

To reach this goal, we need the involvement of your citizens and local officials, ensuring we understand the important local and regional issues and develop a plan that best fits within your community.

## WHAT DO I DO NOW?

Please view the maps showing the proposed bypass alignments.

The first two tables have identical maps showing the main bypass alignments. The second set of tables have identical maps showing the interchange options near I-30.

We encourage you to fill out your comment form at the round tables in the back of the room and leave your comments with us tonight. If you are unable to provide a comment form tonight, the following slide gives additional options to provide input.

Again, our staff is here to help you understand the project and listen to your concerns. We look forward to visiting with you.

## **THANK YOU!**

## Please Submit Your Comments by: Wednesday, February 20, 2019

Leave Your Comment Form Here Tonight or Send the Comment Form Back to:

Garver, Attn: Jon Hetzel, 4701 Northshore Drive, North Little Rock, AR 72118

Email Your Comments to: <a href="mailto:PublicInvolvement@GarverUSA.com">PublicInvolvement@GarverUSA.com</a>

Information is available at:

http://ardot.gov/public\_meetings/2019\_PM/070442/070442.aspx

## **Appendix C** — Agency and Tribal Coordination

#### **Agency Consultation**

Agency consultation letters and exhibits were sent to the following agencies for project coordination. Agency response dates are noted.

Arkansas Department of Environmental Quality

Response received September 25, 2018

Arkansas Department of Health

Response received June 6, 2018

Arkansas Department of Parks and Tourism

Response received July 20, 2018

Arkansas Game and Fish Commission

Response received July 20, 2018

**Arkansas Historic Preservation Program** 

Responses received May 14, 2018

Arkansas Natural Heritage Commission

Responses with species occurrence data received June 12 and July 24, 2018

Arkansas Natural Resources Commission

No response received to date

Arkadelphia Water Utilities

No response received to date

U.S. Department of Agriculture, Natural Resources Conservation Service (NRCS)

Response received May 30, 2018, August 3, 2018, and February 5, 2020

U.S. Fish and Wildlife Service (USFWS)

- Responses received May 23 and July 25, 2018
- IPaC list received January 21, 2020

U.S. Geological Survey (USGS)

No response received to date



Mr. Bill McAbee, Environmental Manager Garver, LLC 4701 Northshore Drive North Little Rock, Arkansas 72118

Re: National Environmental Policy Act (NEPA): Comments Requested Regarding

Arkadelphia Bypass from Hwy. 67 to Hwy 51

Dear Mr. McAbee:

This is in reference to your July 6, 2018 submittal regarding the Arkadelphia Bypass from Hwy. 67 to Hwy 51 in Arkadelphia, Arkansas. Based upon the information submitted, and assuming the Arkansas Department of Transportation (ARDOT) is among the agencies contacted regarding this project, the Department does not have any specific objections at this time.

It is reminded, if the construction site will disturb in excess of one (1) acre, the permittee must comply with the terms of the Stormwater Construction General Permit ARR150000 prior to the start of construction. The ARR150000 does not authorize any activity to be conducted in Waters of the State or Waters of the United States. Any work being conducted in Waters of the State will require a Short Term Activity Authorization (STAA) from ADEQ in accordance with Regulation 2.305. A STAA is necessary for any in-stream activity that may cause an exceedance of applicable water quality standards, including, but not limited to: gravel removal, bridge or crossing repair/maintenance, bank stabilization, debris removal, culvert replacement, flood control projects, and stream relocation. Any work being conducted in Waters of the United States may require a Section 404 permit from the U.S. Army Corps of Engineers. All applicable State and Federal laws must be met before, during and after completion of the project.

This letter is issued in reliance upon the statements and representations made in the submittal and the Department has no responsibility for adequacy or proper functioning of the proposed project. If there are further questions, please contact the Permits Branch at 501-682-0623.

Sincerely,

Bryan Leamons, P.E.

Senior Operations Manager, Office of Water Quality

BL:tl

cc: Audree Miller, Pollution Prevention Coordinator, Office of Law and Policy



### **Arkansas Department of Health**

4815 West Markham Street ● Little Rock, Arkansas 72205-3867 ● Telephone (501) 661-2000 Governor Asa Hutchinson Nathaniel Smith, MD, MPH, Director and State Health Officer

June 6, 2018

Bill McAbee Garver, LLC 4701 Northshore Drive North Little Rock, AR 72118

RE:

Arkadelphia Bypass from Hwy. 67 to Hwy. 51/8

Arkadelphia, Clark County, Arkansas

ARDOT Job No. 070442, Hwy. 67-Hwy. 51 (Arkadelphia Bypass) P.E.

Dear Mr. McAbee,

A staff review has been made of the information received on the following project:

ARDOT Project	ADH Number	Permit Name	County
070442	32-7846	ARKADELPHIA BYPASS HWY 67 TO HWY 51/8	CLARK

The Engineering Section has no comments on the submittal.

If you have any questions or comments, please coordinate them through Tyler Couch at (501) 280-4428.

Sincerely,

Lyle Godfrey, P.E.

Chief, Technical Support

\_ /s\_ fr

**Engineering Section** 

LG:DR:BG:tc

CC:

Arkadelphia Waterworks, 700 Clay Street, Arkadelphia, AR 71923

#### Schmidt, Cassie P.

**Subject:** RE: Arkadelphia Bypass - possible 6(f)3 conflicts

From: Matt McNair < matt.mcnair@arkansas.gov >

Date: July 20, 2018 at 3:57:01 PM MDT

To: "McAbee, William C." < <u>WCMcAbee@GarverUSA.com</u>> Subject: Arkadelphia Bypass - possible 6(f)3 conflicts

Howdy, Bill. Please find attached a very rough (re: kind of crummy) illustration highlighting possible 6(f)3 conflicts with regard to the upcoming Arkadelphia Bypass project.

My notations didn't show up all that well, but it appears to me the two main concerns will be, depending upon the ultimate project area, Feaster Park and River Park. Feaster Park is just west-southwest of Henderson State, and River Park is sited where Hwy 67 hooks north and into town from the southwest. It's down there by the railroad depot.

Both of these parks have LWCF funding in them, and are encumbered under Section 6(f)3 of the LWCF Act. As you can see, there are a two other parks that might be affected (Arkadelphia Central Park and Ouachita River Baseball Park), but to the best of my knowledge, they are not under 6(f)3 encumbrance.

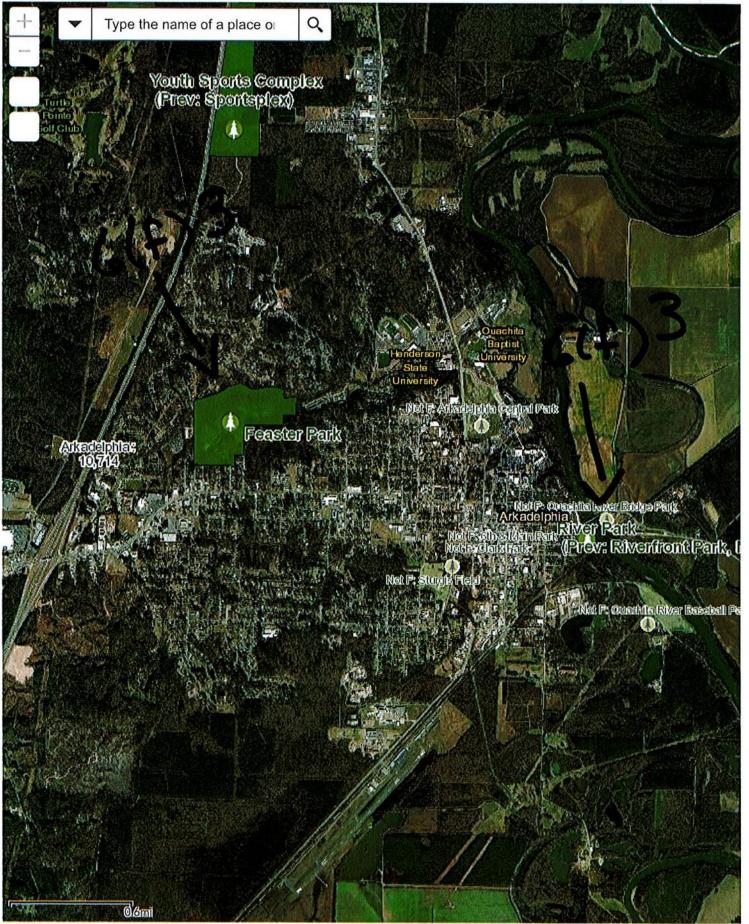
Please accept my apology for the poor quality of this map and the thumbnail-sketch nature of this response; fieldwork has kept me out of the office of late, and promises to keep me out more often than not for a couple more weeks. I just wanted to touch base and give you a general idea of what conflicts might arise. I'll be back in the office more or less regularly starting the week of August 13<sup>th</sup>; if you need anything in the meantime, shoot me an email and I'll get back to you just as soon as I can.

#### Cheers,

Matt McNair
Project Officer / Environmental Review Coordinator
Arkansas Dept. of Parks & Tourism, Outdoor Recreation Grants Program
1 Capitol Mall, Little Rock, AR 72201
501.682.1227
matt.mcnair@arkansas.gov

Appendix C: Agency and Tribal Coordination - Page 5 of 34 with Web AppBuilder for ArcGIS Outdoor GI







Pat Fitts
Director
Caroline Cone
Chief of Staff and
Deputy Director

Chris Colclasure
Deputy Director
Chris Racey
Deputy Director

#### **Arkansas Game and Fish Commission**

July 20, 2018

Bill McAbee Garver, LLC 4701 Northshore Drive North Little Rock, Arkansas 72118

Re: Arkadelphia Bypass from Hwy. 67 to Hwy. 51/8-West Alignment

Arkadelphia, Clark County Arkansas

ARDOT Job No. 070442, Hwy. 67-Hwy. 51 (Arkadelphia Bypass) P.E.

Request for Information-Additional Study Area

Mr. McAbee,

Biologists with the Arkansas Fish and Game Commission (AGFC) have reviewed the west alignment additional study area for the Arkadelphia bypass for potential impacts to fish and wildlife resources of the state. This route should have minimal impacts to known populations of Species of Greatest Conservation Need, however, this project does fall within the consultation zone for Northern Long Eared Bat. Any tree clearing that occurs should happen only after consultation with the United States Fish and Wildlife Service. Any impacts to jurisdictional waters of the United States should be mitigated appropriately, at an approved mitigation bank adhering to the 2008 compensatory mitigation rule.

The AGFC appreciates the opportunity to review this proposed project. Should there be additional questions of the agency, please don't hesitate to contact me. Thanks.

Sincerely,

Justin Stroman

**Environmental Coordination Biologist** 

lustin Stroman



Asa Hutchinson Governor

> Stacy Hurst Director

Arkansas Arts Council

Arkansas Natural Heritage Commission

Arkansas State Archives

Delta Cultural Center

Historic Arkansas Museum

Mosaic Templars Cultural Center

Old State House Museum





1100 North Street Little Rock, AR 72201

(501) 324-9880 fax: (501) 324-9184 tdd: 711

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info@arkansaspreservation.org website:

www.arkansaspreservation.com

An Equal Opportunity Employer

May 14, 2018

Mr. Bill McAbee Garver USA 4701 Northshore Drive North Little Rock, AR 72118

RE: Clark – Arkadelphia

Section 106 Review - FHWA

Arkadelphia Bypass from Hwy. 67 to Hwy 51/8

ARDOT Job No. 070442

Hwy. 67 – Hwy. 51 (Arkadelphia Bypass) P.E.

AHPP Tracking Number: 101106

Dear Mr. McAbee:

This letter is in response to your inquiry regarding properties of archeological, historical, or architectural significance in the area of the proposed referenced project. The staff of the Arkansas Historic Preservation Program (AHPP) has reviewed records pertaining to the area in question.

A records check found several previously recorded archeological sites in the general area of this undertaking. The Arkansas 7/51 Bridge-CL0950 and Missouri Pacific Railroad Depot-CL0193 are listed in the National Register of Historic Places (NRHP) while the Ouachita River Railroad Bridge-CL0121 is eligible for listing. In addition, there are numerous historic structures in the proposed study area that have not been evaluated for eligibility for listing in the NRHP. Because so little work has been undertaken in the proposed study area previously, we recommend that a cultural resources survey be conducted in the area of potential effect (APE).

Tribes that have expressed an interest in the area include the Caddo Nation (Ms. Tamara Francis), the Chickasaw Nation (Ms. Karen Brunso), the Choctaw Nation of Oklahoma (Dr. Ian Thompson), the Osage Nation (Dr. Andrea Hunter), the Quapaw Tribe of Oklahoma (Mr. Everett Bandy), and the Shawnee Tribe of Oklahoma (Ms. Kim Jumper). We recommend that they be consulted in accordance with 36 CFR § 800.2 (c) (2).

Thank you for the opportunity to review this undertaking. Once the undertaking is further along in the planning stages, we look forward to reviewing the updated information. If you should have any questions or comments, please do not hesitate to contact Theresa Russell of my staff at (501)-324-9357.

Sincerely,

Scott Kaufman Director, AHPP

cc: Dr. Andrea Hunter, Osage Nation

Dr. Ann Early, Arkansas Archeological Survey



Asa Hutchinson Governor

> Stacy Hurst Director

Arkansas Arts Council

kansas H

Arkansas Historic Preservation Program

Arkansas State Archives

Delta Cultural Center

\*

Historic Arkansas Museum

Mosaic Templars Cultural Center

Old State House Museum





1100 North Street Little Rock, AR 72201

(501) 324-9619 fax: (501) 324-9618 tdd: 711

e-mail: info@naturalheritage.com website: www.naturalheritage.com Date: June 12, 2018

Subject: Arkadelphia Bypass from Hwy. 67 to Hwy. 51/8

Arkadelphia, Clark County, Arkansas

ANHC No.: P-CF..-18-053

Mr. Bill McAbee Garver, LLC 4701 Northshore Drive North Little Rock, AR 72118

Dear Mr. McAbee:

Staff members of the Arkansas Natural Heritage Commission have reviewed our files for records indicating the occurrence of rare plants and animals, outstanding natural communities, natural or scenic rivers, or other elements of special concern within or near the study area for the Arkadelphia Bypass project from Highway 67 to Highway 51/8. The results of this review are provided as a Geographic Information System (GIS) layer file. Documentation is provided to help you interpret the information in this file.

Our records indicate the potential occurrence of the following species within or near (a one-mile radius of) the project area:

Anguilla rostrata, American eel - State Concern Arcidens wheeleri, Ouachita Rock Pocketbook - Federal Concern (endangered)

Crystallaria asprella, crystal darter - State Concern Cyprogenia sp. cf aberti, Ouachita Fanshell - State Concern Hiodon tergisus, mooneye - State Concern Lampsilis abrupta, Pink Mucket - Federal Concern (endangered) Liodytes rigida, Glossy Swampsnake - State Concern Mugil cephalus, striped mullet - State Concern Nicrophorus americanus. American burying beetle - Federal

*Nicrophorus americanus*, American burying beetle - Federal Concern (endangered)

Percina vigil, saddleback darter - State Concern Pleurobema sintoxia, Round Pigtoe - State Concern Ptychobranchus occidentalis, Ouachita Kidneyshell - State Concern

Theliderma cylindrica, Rabbitsfoot - Federal Concern (threatened)
Toxolasma texasiense, Texas Lilliput - State Concern
Villosa lienosa, little spectaclecase - State Concern

Most of these species were reported from the Ouachita River. Many of these records are older observations. We do not know the current status of these species in this reach of the river. It is of note, that four of the above species are of federal concern. Coordination with the U.S. Fish and Wildlife Service would be appropriate.

A Clark County Element list is enclosed for your reference. Represented on this list are elements for which we have records in our database. The list has been annotated to indicate those elements known to occur within a one and a five-mile radius of the project site. A legend is enclosed to help you interpret the codes used on this list.

Please keep in mind that the project area may contain important natural features of which we are unaware. Staff members of the Arkansas Natural Heritage Commission have not conducted a field survey of the study site. Our review is based on data available to the program at the time of the request. It should not be regarded as a final statement on the elements or areas under consideration. Because our files are updated constantly, you may want to check with us again at a later time.

Thank you for consulting us. It has been a pleasure to work with you on this study.

Sincerely,

Cindy Osborne

Data Manager/Environmental Review Coordinator

Enclosures: GIS Layer file (ANHCDATA)

Clark County Element List (annotated)

Legend Invoice

Cindy Osborne



Asa Hutchinson Governor

> Stacy Hurst Director

Date: July 24, 2018

Subject: Elements of Special Concern

Arkadelphia Bypass-West Alignment

ARDOT Job No.: 070442 Clark County, Arkansas

ANHC No.: P-CF..-18-065

Mr. Bill McAbee Garver 4701 Northshore Drive North Little Rock, AR 72118

Arkansas Arts Council

Arkansas Historic Preservation Program

**Arkansas State Archives** 

**Delta Cultural Center** 

Historic Arkansas Museum

Mosaic Templars **Cultural Center** 

Old State House Museum





1100 North Street Little Rock, AR 72201

(501) 324-9619 fax: (501) 324-9618 tdd: 711

e-mail:

info@naturalheritage.com

website:

www.naturalheritage.com

Dear Mr. McAbee:

Staff members of the Arkansas Natural Heritage Commission have reviewed our files for records indicating the occurrence of rare plants and animals, outstanding natural communities, natural or scenic rivers, or other elements of special concern within or near the following site:

Project Name	County	Quad. Name	Location
Arkadelphia Bypass	Clark	Arkadelphia 7.5'	T17S/R19W/S19,20
West			T17S/R20W/S24

We find no records at present time.

A Clark County Element List is enclosed. Represented on this list are elements for which we have records in our database. The list has been annotated to indicate those elements known to occur within a one and a five mile radius of the project site. A legend is enclosed to help you interpret the codes used on this list.

Please keep in mind that the project area may contain important natural features of which we are unaware. Staff members of the Arkansas Natural Heritage Commission have not conducted a field survey of the study site. Our review is based on data available to the program at the time of the request. It should not be regarded as a final statement on the elements or areas under consideration. Because our files are updated constantly, you may want to check with us again at a later time.

Thank you for consulting us. It has been a pleasure to work with you on this study.

Sincerely,

Cindy Osborne

Data Manager/Environmental Review Coordinator

Enclosures: Legend

Clark County Element List (annotated)

Invoice

## Arkansas Natural Heritage Commission Department of Arkansas Heritage Elements of Special Concern Clark County

	Scientific Name	Common Name	Federal Status	State Status	Global Rank	State Rank
	Animals-Invertebrates					
✓	Alasmidonta marginata	Elktoe	-	INV	G4	S3
	Amblyscirtes belli	Bell's Roadside-Skipper	-	INV	G3G4	S3S4
$\checkmark$	Arcidens wheeleri	Ouachita Rock Pocketbook	LE	SE	G1	S1
	Atrytonopsis hianna	Dusted Skipper	-	INV	G4G5	S2S3
	Beameria venosa	A concealed-tymbal Cicada	=	INV	GNR	S1S2
	Chlosyne gorgone	Gorgone Checkerspot	-	INV	G5	S3
$\checkmark$	Cumberlandia monodonta	Spectaclecase	LE	SE	G3	S2
$\checkmark$	Cyprogenia sp. cf aberti	Ouachita Fanshell	=	INV	GNR	S3
	Euphyes dukesi	Dukes' Skipper	=	INV	G3	S1S2
	Fallicambarus jeanae	Daisie burrowing crayfish	-	INV	G2	S2
	Faxonius menae	Mena crayfish	=	INV	G3	S3
$\checkmark$	Lampsilis abrupta	Pink Mucket	LE	SE	G2	S2
$\checkmark$	Lampsilis ornata	Southern Pocketbook	=	INV	G5	S2
$\checkmark$	Lampsilis powellii	Arkansas Fatmucket	LT	SE	G2	S2
	Lethe creola	Creole Pearly-Eye	-	INV	G3G4	S3
	Microstylum morosum	giant prairie robber fly	-	INV	G3G4	S1
$\checkmark$	Nicrophorus americanus	American burying beetle	LE	SE	G2G3	S1
$\checkmark$	Obovaria olivaria	Hickorynut	-	INV	G4	S3
$\checkmark$	Pleurobema rubrum	Pyramid Pigtoe	-	INV	G2G3	S2
$\checkmark$	Pleurobema sintoxia	Round Pigtoe	-	INV	G4G5	S3
	Poanes yehl	Yehl Skipper	-	INV	G4	S1S3
	Problema byssus	Byssus Skipper	-	INV	G3G4	S3
$\checkmark$	Procambarus parasimulans	Bismark burrowing crayfish	-	INV	G4	S3
$\checkmark$	Ptychobranchus occidentalis	Ouachita Kidneyshell	=	INV	G3G4	S3
	Quadrula fragosa	Winged Mapleleaf	LE	SE	G1	S1
	Quadrula nobilis	Gulf Mapleleaf	-	INV	G4	S3
$\checkmark$	Somatogyrus amnicoloides	Ouachita pebblesnail	=	INV	GX	SX
$\checkmark$	Somatogyrus wheeleri	channelled pebblesnail	-	INV	GX	SX
	Speyeria diana	Diana Fritillary	-	INV	G3G4	S2S3
	Tetraloniella albata	white long-horned bee	-	INV	GNR	S1
$\checkmark$	Theliderma cylindrica	Rabbitsfoot	LT	SE	G3G4	S3
	Toxolasma lividum	Purple Lilliput	-	INV	G3Q	S3
	Toxolasma parvum	Lilliput	-	INV	G5	S3
$\checkmark$	Toxolasma texasiense	Texas Lilliput	-	INV	G4	S3
$\checkmark$	Villosa lienosa	little spectaclecase	-	INV	G5	S3
	Animals-Vertebrates					
✓	Alosa alabamae	Alabama shad	-	INV	G2G3	S1
$\checkmark$	Ambystoma talpoideum	Mole Salamander	-	INV	G5	S3
$\checkmark$	Anguilla rostrata	American eel	-	INV	G4	S3
$\checkmark$	Carphophis amoenus	Common Wormsnake	-	INV	G5	S2
*	Corynorhinus rafinesquii	Rafinesque's big-eared bat	-	INV	G3G4	S3
$\checkmark$	Crystallaria asprella	crystal darter	-	INV	G3	S2
	Erimyzon sucetta	lake chubsucker	-	INV	G5	S3
	Etheostoma clinton	beaded darter	-	INV	GNR	S2
✓	Etheostoma parvipinne	goldstripe darter	-	INV	G4G5	S3

#### Clark Co. (cont.)

Scientific Name	Common Name	Federal Status	State Status	Global Rank	State Rank	
				2115	0.0	
Eurycea paludicola	Western Dwarf Salamander	=	INV	GNR	S3	
Haliaeetus leucocephalus	Bald Eagle	=	INV	G5	S3B,S4	
Hemidactylium scutatum	Four-toed Salamander	=	INV	G5	S2	
Hiodon alosoides	goldeye	=	INV	G5	S2	
Hiodon tergisus	mooneye	=	INV	G5	S2	
Hyla avivoca	Bird-voiced Treefrog	-	INV	G5	S3	
Lethenteron appendix	American brook lamprey	-	INV	G4	S3	
Liodytes rigida	Glossy Swampsnake	-	INV	G5	S3	
Mugil cephalus	striped mullet	-	INV	G5	S2	
Myotis austroriparius	southeastern bat	-	INV	G4	S3	
Myotis septentrionalis	northern long-eared bat	LT	SE	G1G2	S1S2	
Notropis ortenburgeri	Kiamichi shiner	-	INV	G3	S3	
Notropis perpallidus	peppered shiner	-	INV	G3	S3	
Noturus taylori	Caddo madtom	-	INV	G1	S1	
Ophisaurus attenuatus	Slender Glass Lizard	-	INV	G5	S3	
Percina brucethompsoni	Ouachita darter	-	INV	G2?	S2	
Percina uranidea	stargazing darter	-	INV	G3	S2	
Percina vigil	saddleback darter	-	INV	G5	S3	
Picoides borealis	Red-cockaded Woodpecker	LE	SE	G3	S1	
Polyodon spathula	paddlefish	-	INV	G4	S3	
Porphyrio martinicus	Purple Gallinule	-	INV	G5	S1B	
Pteronotropis hubbsi	bluehead shiner	-	INV	G3	S3	
Plants-Vascular						
Agalinis auriculata	ear-leaf false foxglove	-	INV	G3	S1	
Amsonia hubrichtii	Ouachita bluestar	-	INV	G3	S3	
Astragalus crassicarpus var. crassicarpus	purple ground-plum	-	INV	G5T5	S2	
Carex decomposita	cypress-knee sedge	=	INV	G3G4	S2	
Cirsium engelmannii	Engelmann's thistle	-	INV	G4	S1	
Cypripedium kentuckiense	Kentucky lady's-slipper	-	INV	G3	S3	
Diaperia prolifera var. prolifera	big-head rabbit-tobacco	-	INV	G5TNR	S1S3	
Fuirena simplex var. aristulata	western umbrella sedge	-	INV	G5T4	S1	
Glandularia bipinnatifida var. bipinnatifida	Dakota vervain	-	INV	G5T5	S2	
Liatris squarrosa var. squarrosa	hairy scaly blazing-star	-	INV	G5T5	S1	
Lithospermum incisum	fringed puccoon	-	INV	G5	S2S3	
Lithospermum tuberosum	tuberous puccoon	-	INV	G4	S2	
Lycopodiella prostrata	prostrate bog club-moss	-	INV	G5	S1	
Minuartia drummondii	Drummond's sandwort	-	INV	G5	S2S3	
Nemastylis geminiflora	celestial-lily	-	INV	G4	S3	
Physaria gracilis ssp. gracilis	slender bladderpod	-	INV	G5T4	S1	
Plantago rhodosperma	red-seed plantain	-	INV	GNR	S1S2	
Pseudolycopodiella caroliniana	slender bog club-moss	-	INV	G4	S1	
Psilotum nudum	whisk fern	-	INV	G5	S1?	
Pyrrhopappus pauciflorus	few-flower false dandelion	-	INV	G5	S1S2	
Ranunculus flabellaris	yellow water crowfoot	=	INV	G5	S3	
Scleria verticillata	whorled nut-rush	_	ST	G5	S1	
Solidago tortifolia	twist-leaf goldenrod	_	INV	G4G5	S2	
Spiranthes magnicamporum	Great Plains ladies'-tresses	_	INV	G3G4	S1S2	
Spiranthes magnicamporum Spiranthes odorata	fragrant ladies'-tresses	_	INV	G5	S132	
opnaninos odorata	nagrani iaulos -licosco		11 N V	00	01	

#### Clark Co. (cont.)

Scientific Name	Common Name	Federal Status	State Status	Global Rank	State Rank
					_
Spiranthes praecox	giant ladies'-tresses	=	INV	G5	S1S2
Stenosiphon linifolius	false gaura	-	ST	G5	S1
Trichomanes petersii	dwarf bristle fern	-	ST	G4G5	S2
Utricularia inflata	swollen bladderwort	-	INV	G5	S1
Vernonia lettermannii	Letterman's ironweed	-	INV	G3	S3
Viola walteri	Walter's violet	-	INV	G4G5	S1S2
Xyris difformis var. difformis	bog yellow-eyed-grass	-	INV	G5T5	S2
Special Elements-Natural Comm	nunities				
Juniper-Hardwood Woodland		=	INV	GNR	S4
Lowland Pine-Oak Forest		-	INV	GNR	S1
South Central Saline Glade		-	INV	GNR	SNR
West Gulf Coastal Plain Northern Calcareous Prairie		-	INV	GNR	SNR
Special Elements-Other					
Colonial nesting site, water birds		-	INV	GNR	SNR

<sup>\*-</sup>This element of special concern has been recorded within one mile of the Arkadelphia Bypass-West Alignment.

<sup>✓-</sup>These elements of special concern have been recorded within five miles of the Arkadelphia Bypass-West Alignment.

#### **LEGEND**

#### **STATUS CODES**

#### **FEDERAL STATUS CODES**

- C = Candidate species. The U.S. Fish and Wildlife Service has enough scientific information to warrant proposing this species for listing as endangered or threatened under the Endangered Species Act.
- LE = Listed Endangered; the U.S. Fish and Wildlife Service has listed this species as endangered under the Endangered Species Act.
- LT = Listed Threatened; the U.S. Fish and Wildlife Service has listed this species as threatened under the Endangered Species Act.
- -PD = Proposed for Delisting; the U.S. Fish and Wildlife Service has proposed that this species be removed from the list of Endangered or Threatened Species.
- PE = Proposed Endangered; the U.S. Fish and Wildlife Service has proposed this species for listing as endangered.
- PT = Proposed Threatened; the U.S. Fish and Wildlife Service has proposed this species for listing as threatened.
- T/SA = Threatened (or Endangered) because of similarity of appearance. E/SA

#### **STATE STATUS CODES**

- INV = Inventory Element; The Arkansas Natural Heritage Commission is currently conducting active inventory work on these elements. Available data suggests these elements are of conservation concern. These elements may include outstanding examples of Natural Communities, colonial bird nesting sites, outstanding scenic and geologic features as well as plants and animals, which, according to current information, may be rare, peripheral, or of an undetermined status in the state. The ANHC is gathering detailed location information on these elements.
- WAT = Watch List Species; The Arkansas Natural Heritage Commission is not conducting active inventory work on these species, however, available information suggests they may be of conservation concern. The ANHC is gathering general information on status and trends of these elements. An "\*" indicates the status of the species will be changed to "INV" if the species is verified as occurring in the state (this typically means the agency has received a verified breeding record for the species).
- MON = Monitored Species; The Arkansas Natural Heritage Commission is currently monitoring information on these species. These species do not have conservation concerns at present. They may be new species to the state, or species on which additional information is needed. The ANHC is gathering detailed location information on these elements
- SE = State Endangered; this term is applied differently for plants and animals.

Animals – These species are afforded protection under Arkansas Game and Fish Commission (AGFC) Regulation. The AGFC states that it is unlawful to import, transport, sell, purchase, hunt, harass or possess any threatened or endangered species of wildlife or parts. The AGFC lists as endangered any wildlife species or subspecies endangered or threatened with extinction, listed or proposed as a candidate for listing by the U.S. Fish and Wildlife Service or any native species or subspecies listed as endangered by the Commission.

Plants – These species have been recognized by the Arkansas Natural Heritage Commission as being in danger of being extirpated from the state. This is an administrative designation with no regulatory authority.

ST = State Threatened; These species have been recognized by the Arkansas Natural Heritage Commission as being likely to become endangered in Arkansas in the foreseeable future, based on current inventory information. This is an administrative designation with no regulatory authority.

#### **DEFINITION OF RANKS**

Global Ranks

G1 = Critically imperiled globally. At a very high risk of extinction due to extreme rarity (often 5 or fewer populations), very steep declines, or other factors.

G2	=	Imperiled globally. At high risk of extinction due to very restricted range, very few populations (often 20 or fewer), steep declines, or other factors.
G3	=	Vulnerable globally. At moderate risk of extinction due to a restricted range, relatively few populations (often 80 or fewer), recent and widespread declines, or other factors.
G4	=	Apparently secure globally. Uncommon but not rare; some cause for long-term concern due to declines or other factors.
G5	=	Secure globally. Common, widespread and abundant.
GH	=	Of historical occurrence, possibly extinct globally. Missing; known from only historical occurrences, but still some hope of rediscovery.
GU	=	Unrankable. Currently unrankable due to lack of information or due to substantially conflicting information about status or trends.
GX	=	Presumed extinct globally. Not located despite intensive searches and virtually no likelihood of rediscovery.
GNR	=	Unranked. The global rank not yet assessed.
GNA	=	Not Applicable. A conservation status rank is not applicable.
T-RANK	S=	T subranks are given to global ranks when a subspecies, variety, or race is considered at the state level. The subrank is made up of a "T" plus a number or letter (1, 2, 3, 4, 5, H, U, X) with the same ranking rules as a full species.
State I	Ranks	
S1	=	Critically imperiled in the state due to extreme rarity (often 5 or fewer populations), very steep declines, or other factors making it vulnerable to extirpation.
S2	=	Imperiled in the state due to very restricted range, very few populations (often 20 or fewer), steep declines, or other factors making it vulnerable to extirpation.
S3	=	Vulnerable in the state due to a restricted range, relatively few populations (often 80 or fewer), recent and widespread declines, or other factors making it vulnerable to extirpation.
S4	=	Apparently secure in the state. Uncommon but not rare; some cause for long-term concern due to declines or other factors.
<b>S</b> 5	=	Secure in the state. Common, widespread and abundant.
SH	=	Of historical occurrence, with some possibility of rediscovery. Its presence may not have been verified in the past 20-40 years. A species may be assigned this rank without the 20-40 year delay if the only known occurrences were destroyed or if it had been extensively and unsuccessfully sought.
SU	=	Unrankable. Currently unrankable due to lack of information or due to substantially conflicting information about status or trends.
SX	=	Presumed extirpated from the state. Not located despite intensive searches and virtually no likelihood of rediscovery.
SNR	=	Unranked. The state rank not yet assessed.
SNA	=	Not Applicable. A conservation status rank is not applicable.
Genera	I Ranking	Notes
Q	=	A "Q" in the global rank indicates the element's taxonomic classification as a species is a matter of conjecture among scientists.
RANGE	S=	Ranges are used to indicate a range of uncertainty about the status of the element.
?	=	A question mark is used to denote an inexact numeric rank.
В	=	Refers to the breeding population of a species in the state.

Refers to the non-breeding population of a species in the state.



#### **United States Department of Agriculture**

May 30, 2018

Garver, LLC 4701 Northshore Drive North Little Rock, AR 72118

Subject: Farmland Conversion Impact Rating

Garver, LLC

ARDOT Job No. 070442 Hwy 67-Hwy 51

Dear Bill McAbee

This letter is in response to your request for information related to Prime Farmland and Farmland of Statewide Importance for the proposed Arkadelphia Bypass from Hwy 67 to Hwy 51/8 in Clark County, Arkansas. Some areas in the proposed area are considered Prime Farmland or Farmland of Statewide Importance, as showed on the attached maps.

Should you have any questions or need additional information, please call me at (870) 345-3347 or email at marie.ross@ar.usda.gov.

Sincerely,

Marie Ross

Resource Soil Scientist USDA-Natural Resources Conservation Service 640 South 6<sup>th</sup> Street Arkadelphia AR 71923 870-345-3347

**Enclosures:** Form AD-1006 Farmland Classification Map

Edgar Mersiovsky, NRCS, State Soil Scientist, Little Rock, AR



FA	U.S. Departmen				dination - Pa	age 17 of 34		
PART I (To be completed by Federal Agency)  Date Of			Date Of Land Evaluation Request May 25, 2018					
Name of Project Arkadelphia Bypass		gency Involved		,,				
Proposed Land Use construct bypass		County ar	nd State Clark	County, AR	?			
PART II (To be completed by NRCS)		Date Reg	uest Received <b>//ay 10</b> , 20	B)18	Person C Marie	ompleting For	m:	
Does the site contain Prime, Unique, Statew	ide or Local Important Farmland		ES NO	Acres In		Average	Farm Size	
(If no, the FPPA does not apply - do not com		*	$\checkmark$	278,293		777		
Major Crop(s)	Farmable Land In Govt.					Defined in FF	PPA	
soybeans	Acres: 118739 % 2	-VI II		Acres: 11	0,0,,,	21		
Name of Land Evaluation System Used SCS LESA	Name of State or Local S		nent System	Date Land E	∨aluation R	eturned by NF	RCS	
		nsas		1	Alternative	e Site Rating		
PART III (To be completed by Federal Agen	cy)			Site A	Site B	Site Kaling	Site D	
A. Total Acres To Be Converted Directly				635.6				
B. Total Acres To Be Converted Indirectly				0				
C. Total Acres In Site				635.6				
PART IV (To be completed by NRCS) Land	Evaluation Information							
A. Total Acres Prime And Unique Farmland				376				
B. Total Acres Statewide Important or Local	Important Farmland			0				
C. Percentage Of Farmland in County Or Lo	cal Govt. Unit To Be Converted			0.3				
D. Percentage Of Farmland in Govt. Jurisdic	tion With Same Or Higher Relati	ve Value		100				
PART V (To be completed by NRCS) Land Evaluation Criterion Relative Value of Farmland To Be Converted (Scale of 0 to 100 Points)				25.9				
PART VI (To be completed by Federal Agen (Criteria are explained in 7 CFR 658.5 b. For C		CPA-106)	Maximum Points	Site A	Site B	Site C	Site D	
1. Area In Non-urban Use	N - 1 32	20	(15)					
2. Perimeter In Non-urban Use			(10)					
3. Percent Of Site Being Farmed			(20)					
4. Protection Provided By State and Local G	Sovernment		(20)					
5. Distance From Urban Built-up Area			(15)					
6. Distance To Urban Support Services			(15)					
7. Size Of Present Farm Unit Compared To	Average		(10)					
8. Creation Of Non-farmable Farmland			(10)					
9. Availability Of Farm Support Services			(5)					
10. On-Farm Investments			(20)					
11. Effects Of Conversion On Farm Support	Services		(10)					
12. Compatibility With Existing Agricultural U	se		(10)					
TOTAL SITE ASSESSMENT POINTS			160	0	0	0	0	
PART VII (To be completed by Federal Ag	gency)							
Relative Value Of Farmland (From Part V)			100	25.9	0	0	0	
Total Site Assessment (From Part VI above or local site assessment)			160	0	0	0	0	
TOTAL POINTS (Total of above 2 lines)			260	25.9	0	0	0	
Site Selected:	Date Of Selection			Was A Local		sment Used?		
Reason For Selection:			1)	L				
Name of Federal agency representative compl	eting this form: Natural Re	sources	s Conserva	aton Serv	ice D	ate: 5/25/2	2018	

#### STEPS IN THE PROCESSING THE FARMLAND AND CONVERSION IMPACT RATING FORM 34

- Step 1 Federal agencies (or Federally funded projects) involved in proposed projects that may convert farmland, as defined in the Farmland Protection Policy Act (FPPA) to nonagricultural uses, will initially complete Parts I and III of the form. For Corridor type projects, the Federal agency shall use form NRCS-CPA-106 in place of form AD-1006. The Land Evaluation and Site Assessment (LESA) process may also be accessed by visiting the FPPA website, <a href="http://fppa.nrcs.usda.gov/lesa/">http://fppa.nrcs.usda.gov/lesa/</a>.
- Step 2 Originator (Federal Agency) will send one original copy of the form together with appropriate scaled maps indicating location(s)of project site(s), to the Natural Resources Conservation Service (NRCS) local Field Office or USDA Service Center and retain a copy for their files. (NRCS has offices in most counties in the U.S. The USDA Office Information Locator may be found at <a href="http://offices.usda.gov/scripts/ndISAPI.dll/oip\_public/USA\_map">http://offices.usda.gov/scripts/ndISAPI.dll/oip\_public/USA\_map</a>, or the offices can usually be found in the Phone Book under U.S. Government, Department of Agriculture. A list of field offices is available from the NRCS State Conservationist and State Office in each State.)
- Step 3 NRCS will, within 10 working days after receipt of the completed form, make a determination as to whether the site(s) of the proposed project contains prime, unique, statewide or local important farmland. (When a site visit or land evaluation system design is needed, NRCS will respond within 30 working days.
- Step 4 For sites where farmland covered by the FPPA will be converted by the proposed project, NRCS will complete Parts II, IV and V of the form.
- Step 5 NRCS will return the original copy of the form to the Federal agency involved in the project, and retain a file copy for NRCS records.
- Step 6 The Federal agency involved in the proposed project will complete Parts VI and VII of the form and return the form with the final selected site to the servicing NRCS office.
- Step 7 The Federal agency providing financial or technical assistance to the proposed project will make a determination as to whether the proposed conversion is consistent with the FPPA.

#### INSTRUCTIONS FOR COMPLETING THE FARMLAND CONVERSION IMPACT RATING FORM

(For Federal Agency)

Part I: When completing the "County and State" questions, list all the local governments that are responsible for local land use controls where site(s) are to be evaluated.

Part III: When completing item B (Total Acres To Be Converted Indirectly), include the following:

- 1. Acres not being directly converted but that would no longer be capable of being farmed after the conversion, because the conversion would restrict access to them or other major change in the ability to use the land for agriculture.
- 2. Acres planned to receive services from an infrastructure project as indicated in the project justification (e.g. highways, utilities planned build out capacity) that will cause a direct conversion.
- Part VI: Do not complete Part VI using the standard format if a State or Local site assessment is used. With local and NRCS assistance, use the local Land Evaluation and Site Assessment (LESA).
- 1. Assign the maximum points for each site assessment criterion as shown in § 658.5(b) of CFR. In cases of corridor-type project such as transportation, power line and flood control, criteria #5 and #6 will not apply and will, be weighted zero, however, criterion #8 will be weighed a maximum of 25 points and criterion #11 a maximum of 25 points.
- 2. Federal agencies may assign relative weights among the 12 site assessment criteria other than those shown on the FPPA rule after submitting individual agency FPPA policy for review and comment to NRCS. In all cases where other weights are assigned, relative adjustments must be made to maintain the maximum total points at 160. For project sites where the total points equal or exceed 160, consider alternative actions, as appropriate, that could reduce adverse impacts (e.g. Alternative Sites, Modifications or Mitigation).

**Part VII:** In computing the "Total Site Assessment Points" where a State or local site assessment is used and the total maximum number of points is other than 160, convert the site assessment points to a base of 160. Example: if the Site Assessment maximum is 200 points, and the alternative Site "A" is rated 180 points:

 $\frac{\text{Total points assigned Site A}}{\text{Maximum points possible}} = \frac{180}{200} \text{ X } 160 = 144 \text{ points for Site A}$ 

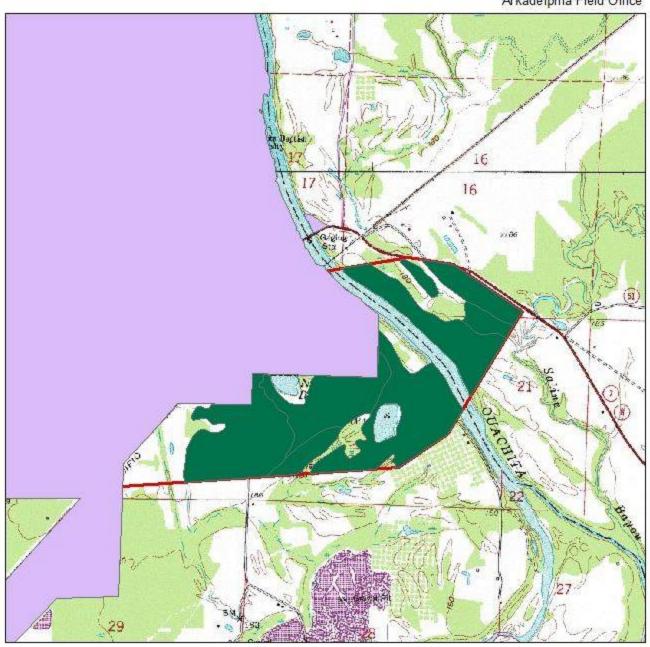
For assistance in completing this form or FPPA process, contact the local NRCS Field Office or USDA Service Center.

NRCS employees, consult the FPPA Manual and/or policy for additional instructions to complete the AD-1006 form.

## Farmland Classification Agency and Tribal Coordination - Page 19 of 34

#### Arkadelphia Bypass from Hwy67 to Hwy 51/8 ARDOT Job No. 070442 Clark County, AR

Assisted by: Marie Ross, RSS Natural Resources Conservation Service Arkadelphia Field Office



Legend 1:24,000 May 2018

City Limit

Farmland Classification

All areas are prime farmland



#### **United States Department of Agriculture**

August 3, 2018

Garver, LLC 4701 Northshore Drive North Little Rock, AR 72118

Attn: Bill McAbee

Subject: FormsAD106

Farmland Conversion Impact Rating ARDot Job 070442 Hwy 67-Hwy 51 P.E

Arkadelphia West Bypass

Clark County, AR

Dear Mr. Bill McAbee

This letter is in response to your request for information related to Prime Farmland and Farmland of Statewide Importance for the proposed Arkadelphia West Bypass located in Clark County, Arkansas. Some areas in the proposed area are considered Prime Farmland or Farmland of Statewide Importance, as showed on the attached maps.

Should you have any questions or need additional information, please call me at (870) 345-3347 or email at marie.ross@ar.usda.gov

Sincerely,

Marie Ross

Resource Soil Scientist USDA-Natural Resources Conservation Service 640 South 6<sup>th</sup> Street

Arkadelphia AR 71923

870-345-3347

cc

Edgar Mersiovsky, Arkansas State Soil Scientist, Little Rock, AR



U.S. DEPARTMENT OF AGRICULTURE Natural Resources Conservation Service NRCS-CPA-106

(Rev. 1-91)

## FARMLAND CONVERSION IMPACT RATING FOR CORRIDOR TYPE PROJECTS

PART I (To be completed by Federal Agency)			3. Date of Land Evaluation Request Sheet 1 of 1					
1. Name of Project Arkadelphia West Byp	ass ARDOT 070422	5. Federal Agency Involved						
2. Type of Project		6. County and State Clark County, AR						
PART II (To be completed by NRCS)		1. Date <b>7/9/</b>	1. Date Request Received by NRCS 2. Person Completing Form Marie Ross					
Does the corridor contain prime, unique statewi     (If no, the FPPA does not apply - Do not complete.)			YES ✓ NO ☐ 4. Acres Irrigated Average Farm Size				Farm Size	
5. Major Crop(s) soybeans	6. Farmable La Acres: 25		nment Jurisdiction % 42	2.6		nt of Farmland As D s:252,989	efined in FPPA % <b>42.6</b>	
Name Of Land Evaluation System Used     SCS/LESA	9. Name of Loc Arkansas	ocal Site Assessment System			10. Date 7/31/	Land Evaluation Re		
PART III (To be completed by Federal Age	ancy)		Alternati	ve Corri	dor For	Segment		
	ancy)		Corridor A	Corri	dor B	Corridor C	Corridor D	
A. Total Acres To Be Converted Directly			36.87	<u> </u>				
B. Total Acres To Be Converted Indirectly, Or T	To Receive Services		0					
C. Total Acres In Corridor			36.87	-				
PART IV (To be completed by NRCS) Lan	nd Evaluation Informatio	n						
A. Total Acres Prime And Unique Farmland			20.6					
B. Total Acres Statewide And Local Important	Farmland		0					
C. Percentage Of Farmland in County Or Loca			0.0042					
D. Percentage Of Farmland in Govt. Jurisdiction	n With Same Or Higher Rela	tive Value	100					
PART V (To be completed by NRCS) Land Eva value of Farmland to Be Serviced or Convert			49.2					
PART VI (To be completed by Federal Agen		Maximum		†			1	
Assessment Criteria (These criteria are expl	5.5	Points						
1. Area in Nonurban Use		15						
2. Perimeter in Nonurban Use		10						
3. Percent Of Corridor Being Farmed		20						
4. Protection Provided By State And Local	Government	20						
<ol><li>Size of Present Farm Unit Compared To</li></ol>	Average	10						
6. Creation Of Nonfarmable Farmland		25	-					
7. Availablility Of Farm Support Services		5		┞				
8. On-Farm Investments		20		ـــــ				
Effects Of Conversion On Farm Support		25		—				
10. Compatibility With Existing Agricultural L	Jse	10		-				
TOTAL CORRIDOR ASSESSMENT POIN	ITS	160	0	0		0	0	
PART VII (To be completed by Federal Age	ency)							
Relative Value Of Farmland (From Part V)		100	49.2	0		0	0	
Total Corridor Assessment (From Part VI abo assessment)	ove or a local site	160	0	0		0	0	
TOTAL POINTS (Total of above 2 lines)		260	49.2	0		0	0	
	cres of Farmlands to be rted by Project:	3. Date Of	Selection:	4. Was	A Local S	ite Assessment Use	ed?	
	,,				YES	□ NO □		
5. Reason For Selection:								
Signature of Person Completing this Part:					DAT	E		
NOTE: Complete a form for each coam		- ^	- 0					

#### **CORRIDOR - TYPE SITE ASSESSMENT CRITERIA**

The following criteria are to be used for projects that have a linear or corridor - type site configuration connecting two distant points, and crossing several different tracts of land. These include utility lines, highways, railroads, stream improvements, and flood control systems. Federal agencies are to assess the suitability of each corridor - type site or design alternative for protection as farmland along with the land evaluation information.

(1) How much land is in nonurban use within a radius of 1.0 mile from where the project is intended? More than 90 percent - 15 points 90 to 20 percent - 14 to 1 point(s) Less than 20 percent - 0 points

(2) How much of the perimeter of the site borders on land in nonurban use? More than 90 percent - 10 points 90 to 20 percent - 9 to 1 point(s) Less than 20 percent - 0 points

(3) How much of the site has been farmed (managed for a scheduled harvest or timber activity) more than five of the last 10 years?

More than 90 percent - 20 points
90 to 20 percent - 19 to 1 point(s)

Less than 20 percent - 0 points

(4) Is the site subject to state or unit of local government policies or programs to protect farmland or covered by private programs to protect farmland?
Site is protected - 20 points

Site is protected - 20 points Site is not protected - 0 points

(5) Is the farm unit(s) containing the site (before the project) as large as the average - size farming unit in the County? (Average farm sizes in each county are available from the NRCS field offices in each state. Data are from the latest available Census of Agriculture, Acreage or Farm Units in Operation with \$1,000 or more in sales.)

As large or larger - 10 points

Below average - deduct 1 point for each 5 percent below the average, down to 0 points if 50 percent or more below average - 9 to 0 points

(6) If the site is chosen for the project, how much of the remaining land on the farm will become non-farmable because of interference with land patterns?

Acreage equal to more than 25 percent of acres directly converted by the project - 25 points

Acreage equal to between 25 and 5 percent of the acres directly converted by the project - 1 to 24 point(s)

Acreage equal to less than 5 percent of the acres directly converted by the project - 0 points

(7) Does the site have available adequate supply of farm support services and markets, i.e., farm suppliers, equipment dealers, processing and storage facilities and farmer's markets?

All required services are available - 5 points

Some required services are available - 4 to 1 point(s)

No required services are available - 0 points

(8) Does the site have substantial and well-maintained on-farm investments such as barns, other storage building, fruit trees and vines, field terraces, drainage, irrigation, waterways, or other soil and water conservation measures? High amount of on-farm investment - 20 points

Moderate amount of on-farm investment - 19 to 1 point(s)

No on-farm investment - 0 points

- (9) Would the project at this site, by converting farmland to nonagricultural use, reduce the demand for farm support services so as to jeopardize the continued existence of these support services and thus, the viability of the farms remaining in the area? Substantial reduction in demand for support services if the site is converted 25 points

  Some reduction in demand for support services if the site is converted 1 to 24 point(s)

  No significant reduction in demand for support services if the site is converted 0 points
- (10) Is the kind and intensity of the proposed use of the site sufficiently incompatible with agriculture that it is likely to contribute to the eventual conversion of surrounding farmland to nonagricultural use?

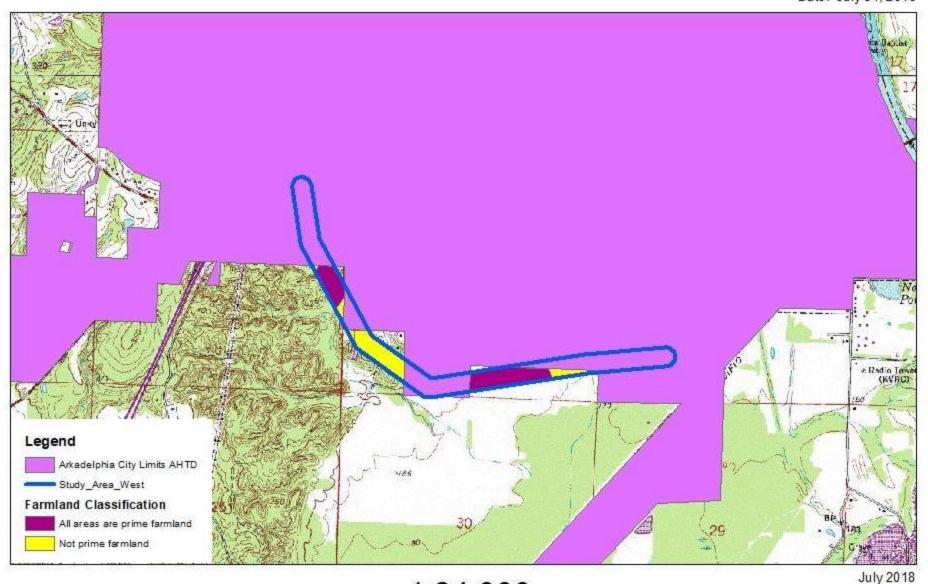
  Proposed project is incompatible to existing agricultural use of surrounding farmland 10 points

  Proposed project is tolerable to existing agricultural use of surrounding farmland 9 to 1 point(s)

  Proposed project is fully compatible with existing agricultural use of surrounding farmland 0 points

# Farmland Classification Arkadelphia Bypass Hwy 67 to Hwy 51/8 West Alignment Clark County, AR ArDOT Job No. 070422

Assisted by: Marie Ross, RSS Arkadelphia Service Center Date: July 31, 2018





#### **United States Department of Agriculture**

February 5, 2020

Garver, LLC 4701 Northshore Drive North Little Rock, AR 72118

Attn: Cassie Schmidt

Subject: Forms AD106

Farmland Conversion Impact Rating

ARDot Job 070422 Clark County, AR

Dear Ms. Schmidt,

This letter is in response to your request for information related to Prime Farmland and Farmland of Statewide Importance for the proposed Arkadelphia Bypass from Hwy 67 to Hwy 51/8 in Clark County, Arkansas. Some areas in the proposed area are considered Prime Farmland or Farmland of Statewide Importance, as showed on the attached maps.

Should you have any questions or need additional information, please call me at (870) 345-3347 or email at marie.ross@usda.gov

Sincerely,

Marie Ross

Resource Soil Scientist USDA-Natural Resources Conservation Service 640 South 6th Street Arkadelphia AR 71923 870-345-3347



U.S. DEPARTMENT OF AGRICULTURE **Natural Resources Conservation Service**  NRCS-CPA-106

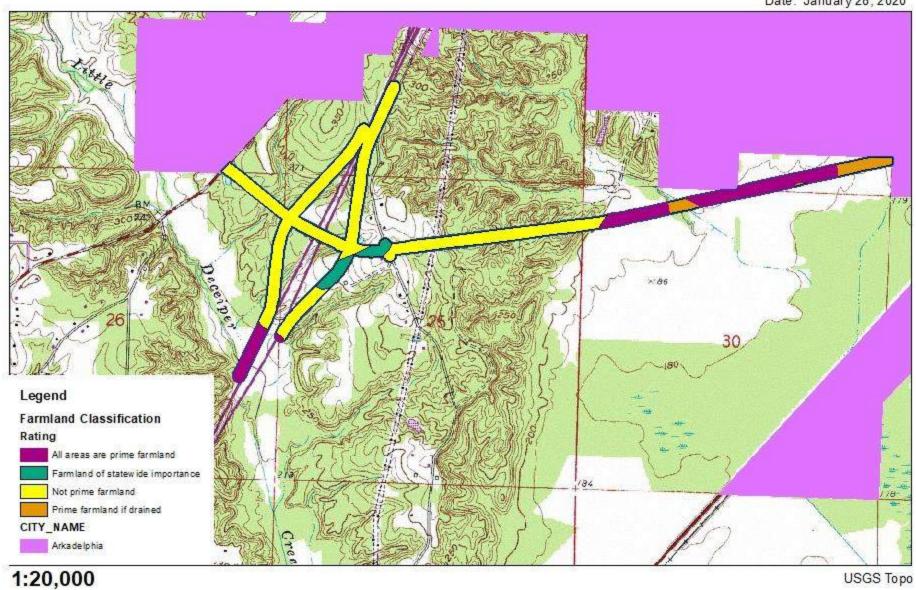
(Rev. 1-91)

#### **FARMLAND CONVERSION IMPACT RATING** FOR CORRIDOR TYPE PROJECTS

PART I (To be completed by Federal Agency)			3. Date of Land Evaluation Request 4. Sheet 1 of 1/13/20					
1. Name of Project Arkadelphia West Bypass ARDOT 070422			5. Federal Agency Involved FHWA					
2. Type of Project New Bypass Facility		6. County and State Clark County, AR						
PART II (To be completed by NRCS)		1. Date 1/1	Request Received by	NRCS	2. Perso <b>Mari</b>	n Completing Form e Ross		
Does the corridor contain prime, unique statewide or local in (If no, the FPPA does not apply - Do not complete additional)	•		YES NO		4. Acres <b>79</b>	Irrigated Average 128	Farm Size	
5. Major Crop(s) soybeans	6. Farmable Lan		nment Jurisdiction % <b>54</b>	.7		nt of Farmland As D s:252,989	efined in FPPA % <b>4</b> 77	
8. Name Of Land Evaluation System Used SCS/LESA	9. Name of Loca <b>Arkansas</b>			••		Land Evaluation Re		
PART III (To be completed by Federal Agency)			Alternati	ve Corri	dor For S	Segment <u>Preferre</u>	ed Alternative	
			Corridor A	Corri	dor B	Corridor C	Corridor D	
A. Total Acres To Be Converted Directly			100.5				<u> </u>	
B. Total Acres To Be Converted Indirectly, Or To Receive S	Services		0					
C. Total Acres In Corridor			100.5					
PART IV (To be completed by NRCS) Land Evaluati	on Information	1						
A. Total Acres Prime And Unique Farmland			21.8					
B. Total Acres Statewide And Local Important Farmland			5.6					
C. Percentage Of Farmland in County Or Local Govt. Unit			0.03					
D. Percentage Of Farmland in Govt. Jurisdiction With Same			100					
PART V (To be completed by NRCS) Land Evaluation Info value of Farmland to Be Serviced or Converted (Scale of			21.9					
PART VI (To be completed by Federal Agency) Corrido		Maximum						
Assessment Criteria (These criteria are explained in 7		Points						
1. Area in Nonurban Use		15						
2. Perimeter in Nonurban Use		10						
3. Percent Of Corridor Being Farmed		20						
4. Protection Provided By State And Local Government		20						
5. Size of Present Farm Unit Compared To Average		10						
6. Creation Of Nonfarmable Farmland		25						
7. Availablility Of Farm Support Services		5						
8. On-Farm Investments		20						
9. Effects Of Conversion On Farm Support Services		25						
10. Compatibility With Existing Agricultural Use		10						
TOTAL CORRIDOR ASSESSMENT POINTS		160	0	0		0	0	
PART VII (To be completed by Federal Agency)								
Relative Value Of Farmland (From Part V)		100	21.9	0		0	0	
Total Corridor Assessment (From Part VI above or a loca assessment)	I site	160	0	0		0	0	
TOTAL POINTS (Total of above 2 lines)		260	21.9	0		0	0	
Corridor Selected:       2. Total Acres of Farm	nlands to be	3. Date Of	Selection:	4. Was	A Local Si	te Assessment Use	d?	
Converted by Projection	ect:							
					YES [	NO 🗌		
5. Reason For Selection:								
Signature of Person Completing this Part:					DATE	Ē		
NOTE: Complete a form for each segment with r	nore than one	Alterna	te Corridor					

#### Farmland Classification Arkadelphia Bypass Hwy 67 to Hwy 51/8 West Alignment Clark County, AR ArDOT Job No. 070422

Assisted by: Marie Ross, RSS NRCS Arkadelphia Service Center Date: January 28, 2020





### United States Department of the Interior

#### FISH AND WILDLIFE SERVICE



May 23, 2018



Mr. Bill McAbee
Environmental Manager
Garver, LLC
4701 Northshore Drive
North Little Rock, Arkansas 72118

Dear Mr. McAbee:

The Service has reviewed your letter and request for information in preparation of National Environmental Policy Act (NEPA) documents for the Arkansas Department of Transportation (ArDOT) related to the Arkadelphia Bypass from U.S. Highway 67 to Arkansas Highway 51/8 near Arkadelphia, Clark County, Arkansas. The request for information is related to development of alternatives for ArDOT Job No. 070442 and was received on May 17, 2018.

Your letter specifically requested review of the proposed study area and to notify you of any constraints or concerns regarding the proposed project, such as unique environmental features or environmentally sensitive areas, socio-economic issues, proposed urban developments, and permit or approvals that should be obtained prior to construction of the project. The Service has reviewed the project location, along with our records and offers the following comments.

As indicated within your letter, the Information for Planning and Consultation System (IPaC), identified a total of 9 threatened, endangered, or candidate species within the vicinity of this action, along with other species of conservation concern. The list includes:

#### **Federally Listed Species**

Northern Long-eared Bat Myotis septentrionalis	Threatened
Piping Plover Charadrius melodus	Threatened
Arkansas Fatmucket Lampsilis powellii	Threatened
Ouachita Rock Pocketbook Arcidens wheeleri	Endangered
Pink Mucket (pearlymussel) Lampsilis abrupta	Endangered
Rabbitsfoot Theliderma cylindrica	Threatened
Spectaclecase (mussel) Margaritifera monodonta	Endangered
Winged Mapleleaf Quadrula fragosa	Endangered
American Burying Beetle Nicrophorus americanus	Endangered

**Critical Habitats** 

There are no critical habitats at this location.

Mr. Bill McAbee

#### **Migratory Birds and Eagles**

Certain birds are protected under the Migratory Bird Treaty Act and the Bald and Golden Eagle Protection Act. Due to the proximity to the Ouachita River and the habitat within this area, it is likely that Bald Eagle, *Haliaeetus leucocephalus*, inhabits this area.

Any person or organization who plans or conducts activities that may result in impacts to migratory birds, eagles, and their habitats should follow appropriate regulations:

- 1. The Migratory Bird Treaty Act of 1918.
- 2. The Bald and Golden Eagle Protection Act of 1940.

Although each species identified will require further assessment and a determination by the action agency, the primary species that could be affected by this action, depending on the alternative, route, and design of the action are the freshwater mussel species identified that exist and have habitats within the Ouachita River. These species and their habitats lie within the project boundaries identified and the potentially affected areas upstream and downstream of the proposed action. Depending on the nature of the action, this project may require consultation in accordance with Section 7 of the Endangered Species Act.

Sediment and/or nutrient transport from the proposed project location may have direct, indirect, and/or cumulative effects to mussels, fish hosts, and/or their habitat(s). The effects of sedimentation and nutrients (e.g., ammonia, etc.) on mussels, fish, and their habitats are well documented in the scientific literature. Adverse effects associated with sedimentation and nutrification from all phases of construction activities may be minimized and/or alleviated through proper implementation and maintenance of erosion control best management practices and maintaining vegetative buffers. Buffer width is dependent upon slope, vegetation type, and soil types. The Service can provide additional technical assistance on appropriate vegetative buffer widths upon request.

The comments herein are for the sole purpose of providing technical assistance to the action agency or for individual pre-project planning assistance. These comments and opinions should not be misconstrued as an "effect determination" or considered as concurrence with any proceeding determination(s) by the action agency in accordance with Section 7 of the ESA. These comments do not authorize the "take" of a threatened or endangered species as defined under the ESA. There are mechanisms available for project proponents to receive authorization from the Service for "incidental take" (e.g., an ESA Section 10 Incidental Take Permit or a Biological Opinion with "incidental take" provisions).

The Service recommends preliminary planning, use of best management practices, and selection of alternatives that would minimize and/or avoid affects to these species. More detailed recommendations and guidance cannot be provided at this time due to the limited project

Mr. Bill McAbee

information available. We look forward to assisting your further as more details of the action become available and alternatives are selected.

For further assistance or if you have any questions, please contact Lindsey Lewis at (501) 513-4489 or lindsey\_lewis@fws.gov.

Sincerely,

Melvin L. Tobin Field Supervisor



## United States Departmendiof the Interior

#### FISH AND WILDLIFE SERVICE

Arkansas Ecological Service Field Office 110 South Amity Road, Suite 300 Conway, Arkansas 72032

July 25, 2018



Mr. Bill McAbee Environmental Manager Garver, LLC 4701 Northshore Drive North Little Rock, Arkansas 72118

Dear Mr. McAbee:

The Service has reviewed your letter and request for information in preparation of National Environmental Policy Act (NEPA) documents for the Arkansas Department of Transportation (ArDOT) related to the Arkadelphia Bypass from U.S. Highway 67 to Arkansas Highway 51/8 - West Alignment near Arkadelphia, Clark County, Arkansas. The request for information is related to development of alternatives for ArDOT Job No. 070442 and was received on July 9, 2018.

Your letter specifically requested review of the proposed study area and to notify you of any constraints or concerns regarding the proposed project, such as unique environmental features or environmentally sensitive areas, socio-economic issues, proposed urban developments, and permit or approvals that should be obtained prior to construction of the project. The Service has reviewed the project location along with our records and offers the following comments.

As indicated within your letter, the Information for Planning and Consultation System (IPaC), identified a total of 8 threatened, endangered, or candidate species within the vicinity of this action along with other species of conservation concern. The list includes:

#### **Federally Listed Species**

Northern Long-eared Bat Myotis septentrionalis	Threatened
Piping Plover Charadrius melodus	Threatened
Ouachita Rock Pocketbook Arcidens wheeleri	Endangered
Pink Mucket (pearlymussel) Lampsilis abrupta	Endangered
Rabbitsfoot Theliderma cylindrica	Threatened
Spectaclecase (mussel) Margaritifera monodonta	Endangered
Winged Mapleleaf Quadrula fragosa	Endangered
American Burying Beetle Nicrophorus americanus	Endangered

**Critical Habitats** 

There are no critical habitats at this location.

Mr. Bill McAbee 2

#### **Migratory Birds and Eagles**

Certain birds are protected under the Migratory Bird Treaty Act and the Bald and Golden Eagle Protection Act. Due to the proximity to the Ouachita River and the habitat within this area, it is likely that the Bald Eagle, *Haliaeetus leucocephalus*, inhabits this area.

Any person or organization who plans or conducts activities that may result in impacts to migratory birds, eagles, and their habitats should follow appropriate regulations:

- 1. The Migratory Birds Treaty Act of 1918.
- 2. The Bald and Golden Eagle Protection Act of 1940.

Although, each species identified will require further assessment and a determination by the action agency, the primary species that could be affected by this action, depending on the alternative, route, and design of the action, are the freshwater mussel species identified that exist and have habitats within the Ouachita River. These species and their habitats lie within the project boundaries identified and the potentially affected areas upstream and downstream of the proposed action. Depending on the nature of the action, this project may require consultation in accordance with Section 7 of the Endangered Species Act.

Sediment and/or nutrient transport from the proposed project location may have direct, indirect, and/or cumulative effects to mussels, fish hosts, and/or their habitat(s). The effects of sedimentation and nutrients (e.g., ammonia, etc.) on mussels, fish, and their habitats are well documented in the scientific literature. Adverse effects associated with sedimentation and nutrification from all phases of construction activities may be minimized and/or alleviated through proper implementation and maintenance of erosion control best management practices and maintaining vegetative buffers. Buffer width is dependent upon slope, vegetation type, and soil types. The Service can provide additional technical assistance on appropriate vegetative buffer widths upon request.

The comments herein are for the sole purpose of providing technical assistance to the action agency or for individual pre-project planning assistance. These comments and opinions should not be misconstrued as an "effect determination" or considered as concurrence with any proceeding determination(s) by the action agency in accordance with Section 7 of the ESA. These comments do not authorize the "take" of a threatened or endangered species, as defined under the ESA. There are mechanisms available for project proponents to receive authorization from the Service for "incidental take" (e.g., an ESA Section 10 Incidental Take Permit or a Biological Opinion with "incidental take" provisions).

The Service recommends preliminary planning, use of best management practices, and selection of alternatives that would minimize and/or avoid affects to these species. More detailed recommendations and guidance cannot be provided at this time due to limited project

Mr. Bill McAbee

information available. We look forward to assisting you further as more details of the action become available and alternatives are selected.

For further assistance or if you have any questions, please contact Lindsey Lewis at (501) 513-4489 or lindsey\_lewis@fws.gov.

Sincerely,

Melvin L. Tobin Field Supervisor

cc: Project File Read File

Filename: C:\Users\lilewis\Documents\PROJECTS\FY2018\ARDOT\Job No. 070442 - Arkadelphia Bypass\AFO Letter- West Alignment - ArDOT Job 070442 - Arkadelphia Bypass - Comments.docx

# **Tribal Consultation**

Tribal consultation letters and exhibits were sent to the following tribes for the project. Tribal response dates are noted.

#### Caddo Nation

• Response received May 24, 2018

#### **Chickasaw Nation**

• No response received to date

#### Choctaw Nation of Oklahoma

• No response received to date

#### Jena Band of the Choctaw Indians

No response received to date

## Osage Nation

• No response received to date

## Quapaw Tribe of Oklahoma

• Response received June 1, 2018

#### Shawnee Tribe of Oklahoma

No response received to date

## Tunica-Biloxi Tribe of Louisiana, Inc.

• No response received to date

# QUAPAW TRIBE OF OKLAHOMA

P.O. Box 765 Quapaw, OK 74363-0765

June 1, 2018

(918) 542-1853 FAX (918) 542-4694

Arkansas State Highway and Transportation Department P.O. Box 2261 Little Rock, Arkansas 72203-2261 JUN 0 4 2018
ENVIRONMENTAL
DIVISION

RECEIVED

Re: ArDOT Job No 070442 Hwy 67, 51 (Arkadelphia Bypass) P.E. Clark County, Arkansas

To whom it may concern,

The Quapaw Tribe Historic Preservation Office has received and reviewed the information provided for the proposed ArDOT Job No 070442 Hwy 67, 51 (Arkadelphia Bypass) P.E. Clark County, Arkansas and concurs with your recommendations for this to conduct a cultural resources survey.

In accordance with the National Historic Preservation Act, (NHPA) [16 U.S.C. 470 §§ 470-470w-6] 1966, undertakings subject to the review process are referred to in S101 (d) (6) (A), which clarifies that historic properties may have religious and cultural significance to Indian tribes. Additionally, Section 106 of NHPA requires Federal agencies to consider the effects of their actions on historic properties (36 CFR Part 800) as does the National Environmental Policy Act (43 U.S.C. 4321 and 4331-35 and 40 CFR 1501.7(a) of 1969).

The Quapaw Tribe has vital interests in protecting its historic and ancestral cultural resources. We do not anticipate that this project will adversely impact any cultural resources or human remains protected under the NHPA, NEPA, or the Native American Graves Protection and Repatriation Act. If however, artifacts or human remains are discovered during project construction, we ask that work cease immediately and that you contact the Quapaw Tribe Historic Preservation Office.

Should you have any questions or need any additional information, please feel free to contact me at the number listed below. Thank you for consulting with the Quapaw Tribe on this matter.

Sincerely,

Tribal Historic Preservation Office

Exerett Bande

Quapaw Tribe of Oklahoma

P.O. Box 765

Quapaw, OK 74363 (w) 918-238-3100

# **Appendix D –** Conceptual Stage Relocation Study

#### CONCEPTUAL STAGE RELOCATION STUDY

Job 070442 Hwy. 67 - Hwy. 51 (Arkadelphia Bypass) P.E. Clark County

September 9, 2020

#### **GENERAL STATEMENT OF RELOCATION PROCEDURE**

Persons displaced as a direct result of acquisition for the proposed project will be eligible for relocation assistance in accordance with Public Law 91-646, the Uniform Relocation Assistance and Real Property Acquisitions Policy Act of 1970, as amended (The Uniform Act). The Relocation Program provides advisory assistance and payments to minimize the adverse impact and hardship of displacement upon such persons. No lawful occupant shall be required to move without receiving a minimum of 90 days advance written notice. All displaced persons; residential, business, farm, nonprofit organization, and personal property relocatees are eligible for reimbursement for actual reasonable moving costs.

It is the Department's Policy that adequate replacement housing will be made available, built if necessary, before any person is required to move from their dwelling. All replacement housing must be fair housing and offered to all affected persons regardless of race, color, religion, sex or national origin. Construction of the project will not begin until decent, safe and sanitary replacement housing is in place and offered to all affected persons.

There are two basic types of residential relocation payments: (1) Replacement Housing payments and (2) Moving Expense payments. Replacement Housing payments are made to qualified owners and tenants. An owner may receive a payment of up to \$31,000.00 for the increased cost of a comparable replacement dwelling. The amount of this payment is determined by a study of the housing market. Owners may also be eligible for payments to compensate them for the increased interest cost for a new mortgage and the incidental expenses incurred in connection with the purchase of a replacement dwelling. A tenant may receive a rental subsidy payment of up to \$7,200.00. Tenants may elect to receive a down payment rather than a rental subsidy to enable them to purchase a replacement dwelling. Replacement Housing payments are made in addition to Moving Expense payments.

Businesses, farms, and nonprofit organizations are eligible for reestablishment payments, not to exceed \$25,000.00. Reestablishment expense payments are made in addition to moving expense payments. A business, farm, or nonprofit organization may be eligible for a fixed payment in lieu of the moving costs and reestablishment costs if relocation cannot be accomplished without a substantial loss of existing patronage. The fixed payment will be computed in accordance with the Uniform Relocation Act and cannot exceed \$40,000.00.

If the displacee is not satisfied with the amounts offered as relocation payments, they will be provided a form to assist in filing a formal appeal. A hearing will be arranged at a time and place convenient for the displacee, and the facts of the case will be promptly and carefully reviewed.

Relocation services will be provided until all persons are relocated or their relocation eligibility expires. The Relocation Office will have listings of available replacement housing and commercial

properties. Information is also maintained concerning other Federal and State Programs offering assistance to displaced persons.

## PROJECT SPECIFIC DISPLACEMENTS

Based on preliminary right-of-way plans and aerial photographs, it is estimated that the alternatives under consideration for the subject project could cause the following displacements and costs:

## **WEST BYPASS ALTERNATIVES**

Alternative A:

No Relocation

Alternative B:

No Relocation

### Alternative H:

	Total	\$ 438.000.00
1	Nonprofit Organization	\$ 40,000.00
5	Businesses	\$ 212,000.00
2	Landlord Businesses	\$ 50,000.00
4	Residential Tenants	\$ 56,000.00
2	Residential Owners	\$ 80,000.00

## **EAST BYPASS ALTERNATIVES**

## Alternative D:

1 2 4 9 1	Residential Owner Residential Tenants Landlord Businesses Businesses Nonprofit Organization Total	\$ \$ \$ \$ \$ \$ \$	40,000.00 24,000.00 100,000.00 360,000.00 40,000.00 <b>564,000.00</b>
	Alternative F:		
1	Residential Owner	\$	40,000.00
2	Residential Tenants	\$	24,000.00
2	Landlord Businesses	\$	50,000.00
2	Businesses	\$	83,000.00
	Total	\$	197,000.00
	Alternative G:		
1	Residential Owner	\$	40,000.00
2	Businesses	\$	83,000.00
	Total	\$	123,000.00

## **INTERCHANGE ALTERNATIVES**

Alternative 1:

	<u> </u>	<del></del>	
18	Residential Tenants	\$	252,000.00
1	Landlord Business	\$	25,000.00
4	Businesses	\$	166,000.00
	Total	\$	443,000.00
	Alternative 1	<u>A:</u>	
1	Business	\$	40,000.00
	Total	\$	40,000.00
	Alternative 2	<u>2:</u>	
1	Business	\$	40,000.00
	Total	\$	40,000.00

The general characteristics of the displacees to be relocated are listed on the Conceptual Stage Relocation Inventory forms in the back of this report.

Alternative 3:

42,000.00

25,000.00

\$ 212,000.00

\$ 279,000.00

\$

Residential Tenants

Landlord Business

Businesses

Total

3

1

5

An available housing inventory has been compiled and it indicates there are at least 51 comparable replacement dwellings available for sale, 19 residential lots available for sale, and 10 comparable replacement dwellings available for rent within a reasonable proximity of the project area. At least nine developed commercial properties and three vacant land commercial properties are currently for sale in the project area. There are no known commercial properties for lease at the time of this report. A breakdown of the available properties is as follows:

Number of
Units
5
11
14
21
51
10
10 3

Residential	
(Monthly Rent) \$ 0.00 - 500.00	4
501.00 and up	6
Total	10
iotai	10
Commercial Properties	
(For Sale)	
\$ O - 100,000	1
100,001 - 200,000	
200,001 - 300,000	3
300,001 - 400,000	0
401,000 and up	2 3 0 3
Total	9
Commercial Land	
(For Sale)	
\$ 0 - 100,000	0
100,001 - 200,000	1
200,001 and up	1 2 3
Total	3
Commercial Properties	
(For Lease)	
\$ 0 - 1,000	0
1,001 - 2,000	Ö
2,001 and up	Ö
Total	0

This is a new location bypass project in Arkadelphia, AR. The dwellings and number of dwellings are comparable and adequate to provide replacement housing for the families displaced on the project. The housing market should not be detrimentally affected and there should be no problems with insufficient housing at this time. In the event housing cannot be found or can be found but not within the displacees' economic means at the time of displacement, Section 206 of Public Law 91-646 (Housing of Last Resort) will be utilized to its fullest and practical extent.

The replacement property inventory was compiled from data obtained from real estate companies and web sites for the subject area. The dwellings contained in the inventory have been determined to be comparable and decent, safe, and sanitary. The locations of the comparable dwellings are not less desirable in regard to public utilities and public and commercial facilities, are reasonably accessible to the displacees' places of employment, adequate to accommodate the displacees, and in neighborhoods which are not subject to unreasonable adverse environmental factors. It has also been determined that the available housing is within the financial means of the displacees and is fair housing open to all persons regardless of race, color, sex, religion or national origin consistent with the requirements of 49 CFR, Subpart A, Section 24.2 and Title VIII of the Civil Rights Act of 1968.

A commercial property inventory indicates there are at least nine developed properties and three vacant lots available in the subject area at this time. The businesses displaced on the project may not be able to relocate in the immediate area of their displacement resulting in termination of the operation. However, in order to assist the displaced businesses and nonprofit organizations in

relocating, the State will explore all possible sources of funding or other resources that may be available to businesses and nonprofit organizations. Sources that will be considered include: State and Local entities, the Department of Housing and Urban Development, the Economic Development Administration, the Small Business Administration, and other Federal Agencies. Emphasis will be given in providing relocation advisory services to the businesses and nonprofit organizations. Appropriate measures will be taken to ensure that each entity displaced is fully aware of their benefits, entitlements, courses of action that are open to it, and any special provisions designed to encourage businesses and nonprofit organizations to relocate within the same community.

All displacees will be offered relocation assistance under provisions in the applicable FHWA regulations. At the time of displacement another inventory of available housing in the subject area will be obtained and an analysis of the market made to ensure that there are dwellings adequate to meet the needs of all displacees. Also, special relocation advisory services and assistance will be administered commensurate with displacees' needs, when necessary. Examples of these include, but are not limited to, Housing of Last Resort as previously mentioned and consultation with local officials, social and federal agencies, and community groups.

There are no other identified unusual conditions involved with this project.

# **CONCEPTUAL STAGE RELOCATION INVENTORY**

Job No.: 070442 Job Name: Hwy. 67 - Hwy. 51 (Arkadelphia Bypass) P.E. Date of Inventory: September 9, 2020

Relo.	Relo. Type <sup>1</sup>	Description	Address	Eligible Costs <sup>2</sup>	Property Value Estimate <sup>3</sup>	# Employees	Occ. Length (Yrs)		
	West Bypass Alternative H								
1	LLBus + ResTen	Mobile Home	369 Red Hill Rd	\$39,000	\$15,000				
2	ResOwn	Home	379 Red Hill Rd	\$40,000	\$34,000				
3	ResOwn	Home	429 Red Hill Rd	\$40,000	\$188,550				
4-6	LLBus + ResTen	Portion of Cox Mobile Manor with 3 residential tenant relocations	3027 Pine St	\$67,000	\$28,500*	1	60		
7	NPOrg	Grace Fellowship Church	469 Red Hill Rd	\$40,000	\$2,800,000	1	40		
8	Bus	Arkadelphia Physical Therapy	3030 Pine St	\$40,000	\$785,000	8	19		
9	Bus	Lucky Liquor	3035 Pine St	\$46,000	\$275,000	5	8		
10	Bus	Private Mechanic Garage	24 Red Hill Rd	\$46,000	\$240,000	3 - 4	5		
11	Bus	AT&T Store	3032 Pine St	\$40,000	\$315,000	Unk	Unk		
12	Bus	Dimensions Sound & Vision	280 Professional Park Dr	\$40,000	\$320,000	2	18		
		Eas	t Bypass Alternative D						
1	LLBus + ResTen	Home	924 Main St	\$37,000	\$45,000				
2	LLBus	Home (Vacant)	925 Clay St	\$25,000	\$42,900				
3	LLBus + ResTen	Home	922 Crittenden St	\$37,000	\$35,000				
4	ResOwn	Home	529 S 9th St	\$40,000	\$182,450				
5	Bus	KFC	921 Clinton St	\$40,000	\$250,000	18	1		
6	LLBus + NPOrg	Grace Bible Church	922 Main St	\$65,000	\$65,000	Unk	Unk		
7	Bus	Mary & Martha's Florist & Gifts	921 Main St	\$40,000	\$220,000	3-4	13		

Relo. #	Relo. Type <sup>1</sup>	Description	Address	Eligible Costs <sup>2</sup>	Property Value Estimate <sup>3</sup>	# Employees	Occ. Length (Yrs)
8	Bus	Ark. Dept. Community Corrections	911 Main St	\$40,000	\$135,000	Unk	Unk
9	Bus	Kidsource Therapy (Vacant)	Unknown	\$40,000	\$70,650	0	Unk
10	Bus	U-Lok-It Storage (Vacant & Listed for Sale for \$289k)	921 Clay St	\$40,000	\$70,500	0	
11	Bus	Lane Refrigeration	903 Carpenter St	\$40,000	\$159,400	Unk	Unk
12	Bus	Security Warehouse (3 western-most buildings relocated)	725 S 7th St	\$40,000	\$288,200	2	20
13	Bus	NAPA Autoparts	922 Clinton St	\$40,000	\$239,050	3-4	15
14	Bus	Stewarts Auto Sales, Inc.	102 N 10th St.	\$40,000	\$66,700	1	24
		Eas	st Bypass Alternative F				
1	LLBus + ResTen	Home	117 Hemphill Rd	\$37,000	\$6,800		
2	LLBus + ResTen	Home (eastern-most only)	108 Clay St	\$37,000	\$130,000		
3	ResOwn	Home	111 S 1st St	\$40,000	\$46,000		
4	Bus	Wade's Cabinet Shop	921 Highway 67 S	\$43,000	\$350,000	2	43
5	Bus	Unnamed commercial complex (Vacant)	Unk	\$40,000	\$43,100	0	Unk
		Eas	st Bypass Alternative G				
1	ResOwn	Home	727 C St	\$40,000	\$37,000		
2	Bus	Wade's Cabinet Shop	921 Highway 67 S	\$43,000	\$350,000	2	43
3	Bus	Unnamed commercial complex (Vacant)	Unknown	\$40,000	\$43,100	0	Unk
		Inte	erchange Alternative 1				
1	Bus	Lucky Liquor	3035 Pine St	\$46,000	\$275,000	5	8
2	Bus	Exxon / Blackmon Oil Co.	3036 Pine St	\$40,000	\$406,000	11	8
3	Bus	Andy's Restaurant	2927 Pine St	\$40,000	\$365,000	Unk	Unk

Relo. #	Relo. Type <sup>1</sup>	Description Address		Eligible Costs <sup>2</sup>	Property Value Estimate <sup>3</sup>	# Employees	Occ. Length (Yrs)
4	Bus	Citgo	3039 Pine St	\$40,000	\$350,000	3	1
5-22	LLBus + ResTen	Portion of Cox Mobile Manor with 18 residential tenant relocations	3027 Pine St	\$277,000	\$1,500,000	1	60
		Inte	rchange Alternative 1A				
1	Bus	Exxon / Blackmon Oil Co.	3036 Pine St	\$40,000	\$406,000	11	8
		Inte	erchange Alternative 2				
1	Bus	Exxon / Blackmon Oil Co.	3036 Pine St	\$40,000	\$406,000	11	8
		Inte	erchange Alternative 3				
1-3	LLBus + ResTen	Portion of Cox Mobile Manor with 3 residential tenant relocations	3027 Pine St	\$67,000	\$28,500	1	60
4	Bus	Arkadelphia Physical Therapy	3030 Pine St	\$40,000	\$785,000	8	19
5	Bus	Lucky Liquor	3035 Pine St	\$46,000	\$275,000	5	8
6	Bus	Private Mechanic Garage	24 Red Hill Rd	\$46,000	\$240,000	3 - 4	5
7	Bus	Dimensions Sound & Vision	280 Professional Park Dr	\$40,000	\$320,000	2	18
8	Bus	AT&T Store	3032 Pine St	\$40,000	\$315,000	Unk	Unk

<sup>1</sup> Relo. Type:

ResOwn Residential Owner ResTen Residential Tenant LLBus **Landlord Business** 

Bus **Business** 

**NPOrg** Nonprofit Organization

<sup>&</sup>lt;sup>2</sup> Replacement Housing Payment, Rental Assistance Payment, Reestablishment Payment, Search Expense Payment, &/or Moving Costs. NOTE: For those displacements involving a mobile home, car lift (mechanic garage), or shipping container (Lucky Liquor), an additional estimated cost of \$3,000 per house/lift/container was applied to the moving cost.

<sup>&</sup>lt;sup>3</sup> Based on ROW cost estimates, county parcel data, and/or estimated cost per acre for last reported sales price.

## **CONCEPTUAL STAGE RELOCATION INVENTORY**

Job No.: 070442 Job Name: Hwy. 67 - Hwy. 51 (Arkadelphia Bypass) P.E. Date of Inventory: September 8, 2020

## West Bypass Alternative A

Relocation Type	Number	Property Values or Residential Rental Rates <sup>1</sup>	Employees Affected (Range)
Residential Owners	0		
Residential Tenants	0		
Landlord Businesses	0		
Businesses	0		
Nonprofit Organizations	0		
Totals	0	N/A	0

<sup>&</sup>lt;sup>1</sup> Based on interviews, ROW cost estimates, county parcel data, sales price, and/or estimated cost per acre for last reported sales price.

# West Bypass Alternative B

Relocation Type	Number	Property Values or Residential Rental Rates <sup>1</sup>	Employees Affected (Range)
Residential Owners	0		
Residential Tenants	0		
Landlord Businesses	0		
Businesses	0		
Nonprofit Organizations	0		
Totals	0	N/A	0

<sup>&</sup>lt;sup>1</sup>Based on interviews, ROW cost estimates, county parcel data, sales price, and/or estimated cost per acre for last reported sales price.

# West Bypass Alternative H

Relocation Type	Number	Property Values or Residential Rental Rates <sup>1</sup>	Employees Affected (Range)
Residential Owners	2	\$30,000.00 - \$261,000.00	N/A
Residential Tenants	4	\$125.00 - \$725.00	N/A
Landlord Businesses	2	\$7,200.00 - \$1,5000,000.00	1 - 2
Businesses	5	\$93,000.00 - \$785,000.00	1 - 8
Nonprofit Organizations	1	\$2,800,000.00	1
Totals	12	N/A	3 - 11

<sup>&</sup>lt;sup>1</sup>Based on interviews, ROW cost estimates, county parcel data, sales price, and/or estimated cost per acre for last reported sales price.

# East Bypass Alternative D

Relocation Type	Number	Property Values or Residential Rental Rates <sup>1</sup>	Employees Affected (Range)
Residential Owners	1	\$181,000.00 - \$243,000.00	N/A
Residential Tenants	2	\$850.00 - \$950.00	N/A
Landlord Businesses	4	\$35,000.00 - \$90,000.00	Unknown
Businesses	9	\$65,000.00 - \$480,000.00	0 - 4
Nonprofit Organizations	1	\$53,000.00 - \$65,000.00	Unknown
Totals	14	N/A	0 - 4

<sup>&</sup>lt;sup>1</sup>Based on interviews, ROW cost estimates, county parcel data, sales price, and/or estimated cost per acre for last reported sales price.

# East Bypass Alternative F

Relocation Type	Number	Property Values or Residential Rental Rates <sup>1</sup>	Employees Affected (Range)
Residential Owners	1	\$46,000.00 - \$97,000.00	N/A
Residential Tenants	2	\$750.00 - \$1,000.00	N/A
Landlord Businesses	2	\$5,000.00 - \$130,000.00	Unknown
Businesses	2	\$27,000.00 - \$350,000.00	0 - 2
Nonprofit Organizations	0		
Totals	5	N/A	0 - 2

<sup>&</sup>lt;sup>1</sup>Based on interviews, ROW cost estimates, county parcel data, sales price, and/or estimated cost per acre for last reported sales price.

# East Bypass Alternative G

Relocation Type	Number	Property Values or Residential Rental Rates <sup>1</sup>	Employees Affected (Range)
Residential Owners	1	\$5,000.00 - \$49,000.00	N/A
Residential Tenants	0		N/A
Landlord Businesses	0		
Businesses	2	\$27,000.00 - \$350,000.00	0 - 2
Nonprofit Organizations	0		
Totals	3	N/A	0 - 2

<sup>&</sup>lt;sup>1</sup>Based on interviews, ROW cost estimates, county parcel data, sales price, and/or estimated cost per acre for last reported sales price.

## Interchange Alternative 1

Relocation Type	Number	Property Values or Residential Rental Rates <sup>1</sup>	Employees Affected (Range)
Residential Owners	0		N/A
Residential Tenants	18	\$125.00 - \$135.00	N/A
Landlord Businesses	1	1,500,000.00	1
Businesses	4	\$272,000.00 - \$406,000.00	3 - 11
Nonprofit Organizations	0		
Totals	22	N/A	1 - 11

<sup>&</sup>lt;sup>1</sup>Based on interviews, ROW cost estimates, county parcel data, sales price, and/or estimated cost per acre for last reported sales price.

# Interchange Alternative 1A

Relocation Type	Number	Property Values or Residential Rental Rates <sup>1</sup>	Employees Affected (Range)
Residential Owners	0		N/A
Residential Tenants	0		N/A
Landlord Businesses	0		
Businesses	1	\$406,000.00	11
Nonprofit Organizations	0		
Totals	1	N/A	11

<sup>&</sup>lt;sup>1</sup> Based on interviews, ROW cost estimates, county parcel data, sales price, and/or estimated cost per acre for last reported sales price.

# Interchange Alternative 2

Relocation Type	Number	Property Values or Residential Rental Rates <sup>1</sup>	Employees Affected (Range)
Residential Owners	0		N/A
Residential Tenants	0		N/A
Landlord Businesses	0		
Businesses	1	\$406,000.00	11
Nonprofit Organizations	0		
Totals	1	N/A	11

<sup>&</sup>lt;sup>1</sup> Based on interviews, ROW cost estimates, county parcel data, sales price, and/or estimated cost per acre for last reported sales price.

# Interchange Alternative 3

Relocation Type	Number	Property Values or Residential Rental Rates <sup>1</sup>	Employees Affected (Range)
Residential Owners	0		N/A
Residential Tenants	3	\$125.00 - \$135.00	N/A
Landlord Businesses	1	\$29,000.00 - \$1,500,000.00	1
Businesses	5	\$93,000.00 - \$785,000.00	1 - 8
Nonprofit Organizations	0		
Totals	8	N/A	1 - 8

<sup>&</sup>lt;sup>1</sup>Based on interviews, ROW cost estimates, county parcel data, sales price, and/or estimated cost per acre for last reported sales price.